## GEO TECHNICAL INVESTIGATION REPORT

### Project:

Proposed Construction of High-rise Buildings at Mallapur, Hyderabad

### Client:

M/s B & C Estates, Hyderabad

August 2009

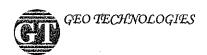
Prepared by:



### GEOMEGHNOLOGIES

# 5-83/ B, V.V Nagar Street No 8,Habsiguda Hyderabad-500 007 Tel:42217757,Cell:9347275255

E-mail:nvenkatrao2005@yahoo.com



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GEOTECHNICAL CONSULTANTS:

**GEO TECHNOLOGIES** 

# 5-83/B, V. V. NAGAR

HABSIGUDA, STREET No. 8

HYDERABAD - 500 007

Tele/Fax: 040 - 27175255

Email: nvenkatrao2005@yahoo.com



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### 1. INTRODUCTION

M/s B & C Estates, Mallapur, are proposing to construct High-rise Buildings at Mallapur, Hyderabad.

The buildings comprise two cellars + 14 upper floors.

Soil investigations were carried out for this project by drilling eight (8) bore holes, conducting Standard Penetration Tests, Sample collection and Laboratory testing. The results of these investigations and recommendations are presented in this Report.

Fig.1 gives the Site Plan of the proposed construction, showing the locations of Bore holes.

The aim of this Report is to determine the type and depth of foundations and the Safe Bearing Capacity based on Field and Laboratory Investigations.



### 2. SCOPE OF WORK

M/s Geo Technologies, Hyderabad, have been awarded the work of Soil Investigation for B & C Estates, Opp: Noma Function Hall, Mallapur, Hyderabad, along with payment of advance Dated 11-07-2009. The following is the Scope of work:

- a) To drill eight (8) bore holes at specified locations.
- b) To conduct Standard Penetration Tests (SPT) as per the IS Code 2131.
- c) To obtain soil / rock samples from the bore holes at significant depths.
- d) To conduct relevant Laboratory tests on the samples for engineering properties.
- e) To determine the safe bearing capacity and depth of foundations.
- f) To prepare a Technical Report incorporating the relevant information and recommendations.

All sub soil investigations are carried out in accordance with the relevant BIS (IS) Codes.



### 3. GEOLOGICAL SET UP

The geology of Hyderabad region comprises the Peninsular gneissic complex of the Archean age.

The major rock types include pink and grey granites, granitic gneisses, migmatites, pegmatites, quartz veins and dolerite dykes. They occur in the form of domes, scarps, massive columnar blocks and 'tors', scattered over a partly flat and partly undulating country. EW & NS trending dolerite dykes and quartz & pegmatite reefs and veins are intruded into granites and gneisses.

The rock type in the site under investigation is granite. The major minerals are quartz, feldspars, and biotite.

### Weathered Zone:

As a result of natural weathering processes, granites and gneisses are broken and fractured, giving rise to inter-connected fissures, which permit percolation of water through the rock. The percolating water promotes further disintegration and decomposition of the rock. Ultimately, the rock is converted to soil, made up of a coarse granular material, which consists of gravel, sand, silt and clay fractions in its constitution. This soil is locally referred to as morum.



### Soils:

The soils in the granitic terrain thus are mainly derived from residual or 'in-situ' weathering of the parent bed rock. Red soils, comprising loamy sand, sand, sandy clay, and derived from granites and gneisses, are the predominant types of soils in this region.

### Local Geology:

The rock formation is predominantly grey granite, belonging to the group of Peninsular Granites.

The surface formation essentially consists of filling, clayey sand and Silty / Hard morum.



### 4. FIELD INVESTIGATIONS

### **OBJECTIVE**:

The sub soil investigation was carried out to determine the nature of stratum and engineering properties of soil which may affect the mode of construction of the proposed structures.

### **BORE HOLES:**

Eight (8) bore holes were drilled at the locations shown in Site Plan (Fig.1). These are designated as BH-1 to BH-8, respectively.

Table-1 gives the details of bore holes.

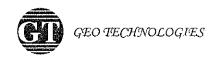
The bore holes were planned so as to yield complete information in the effective and critical zones under the foundations.

### DRILLING:

Rotary Drilling was performed as per IS: 1892. The size of the casing used was 125 to 75 mm yielding samples of NX size.

The following information was collected during the drilling operations:

- Nature of strata
- Details of samples / rock cores
- Water colour
- Rate of drilling



### **STANDARD PENETRATION TEST (SPT):**

Standard Penetration Tests were conducted at frequent intervals in the bore holes. These tests were performed as specified in IS:2131-1981. In this test, a standard weight is dropped through a specified height, and the number of blows required for a penetration of 300 mm is recorded. The number of blows is designated as N. The first 150 mm penetration is neglected for the purpose of seating load. If required, correction is applied to the recorded N value for dilatancy and overburden effects. Also, if the number of blows exceeds 50, Refusal is said to have been reached and further testing is discontinued.

### **FIELD BORE LOGS:**

All the details collected from the field operations are presented in Logs of Bore holes in Annexure at the end of this Report. These logs contain depth wise strata details, drilling rate, results of Standard Penetration Tests and colour of water etc.

### **SAMPLES:**

All the samples collected from the bore holes were properly packed, labeled and transported to Geo Technologies Soil Testing Laboratory at Hyderabad.



### 5. LABORATORY TESTING

The samples were tested at our Geo Technical Laboratory of GEO TECHNOLOGIES at Hyderabad.

The following tests were performed on soil samples:

- Grain size analysis
- Atterberg Limits (LL, PL & PI)
- Specific gravity
- Bulk Density
- > Shear tests

All the tests were conducted in accordance with IS: 2720 (Methods of Tests for Soils).

The following tests were conducted on rock samples:

- Specific Gravity
- Porosity
- Water absorption
- Unconfined compressive test

The tests were performed as per IS: 1124-1974 and IS: 9143-1979.



### 6. RESULTS

Fig. 2 gives the Logs of the bore holes, showing the sub soil profile. The variation of strata with depth is indicated.

Table-2 gives the results of tests on soil samples from bore holes.

Table-3 gives the Rock characteristics of granite which is the main rock type in the site.

Table-4 gives the results of tests on Rock Cores from bore holes.



### 7. SUBSOIL PROFILE & FOUNDATIONS

Based on eight (8) Bore logs, the subsoil profile at the Picket Nallah site is generalized as follows:

### BLOCK-B (BH-1 to BH-4)

Depth (m)	<u>Strata</u>	<u>N Value</u>
0.00 -4.00	Filling / clayey sand / Silty morum	8 – 31
4.00 – 5.50	Hard morum	> 50
Below 5.50	Soft Disintegrated Rock (SDR)	> 50

### BLOCK-A (BH-5 to BH-8)

Depth (m)	<u>Strata</u>	<u>N Value</u>
0.00 –5.00	Filling / Silty clay / Silty morum	10 - 47
5.00 – 6.00	Hard morum	> 50
Below 6.00	Soft Disintegrated Rock (SDR)	> 50

It should be noted that in BH-1, Hard rock boulder exists from 3.5 m depth. In BH-2 (Block-B) and BH-6 (Block-A), Soft Disintegrated Rock (SDR) is seen only from 7.5 m depth



In general, below the proposed sub-cellar floor level (about 6 m depth), the stratum comprises Soft Disintegrated Rock (SDR), which is fairly compact and hard. N values in Soft Disintegrated Rock consistently exceed 50, showing refusal.

The Soft Disintegrated Rock (SDR) is weathered and fissured. Significant portions of the rock show discolouration and weathering effects. Feldspar grains are dull. The rock has dull sound under hammer and shows significant loss of strength as compared with fresh rock. No cores were obtained in SDR strata due to fissured nature of strata.

Water table is encountered in the bore holes at 2.6 - 4.5 m.

Keeping the above points in view, open foundations are recommended resting in SDR strata.



### 8. RECOMMENDATIONS

The following recommendations are made for the proposed construction of Highrise Building in B & C Estates at Mallapur, Hyderabad.

These recommendations are based on Field investigations and Laboratory Tests on samples from eight (8) Bore Holes.

a) The generalized sub soil profile at the site is as follows:

### BLOCK-B (BH-1 to BH-4)

Depth (m)	<u>Strata</u>	<u>N Value</u>
0.00 -4.00	Filling / clayey sand / Silty morum	8 – 31
4.00 – 5.50	Hard morum	> 50
Below 5.50	Soft Disintegrated Rock (SDR)	> 50

### BLOCK-A (BH-5 to BH-8)

Depth (m)	<u>Strata</u>	<u>N Value</u>
0.00 –5.00	Filling / Silty clay / Silty morum	10 - 47
5.00 – 6.00	Hard morum	> 50
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It should be noted that in BH-1, Hard rock boulder exists from 3.5 m depth. In BH-2 (Block-B) and BH-6 (Block-A), Soft Disintegrated Rock (SDR) is seen only from 7.5 m depth

- b) In general, below the proposed sub-cellar floor level (about 6 m depth), the stratum comprises Soft Disintegrated Rock (SDR), which is fairly compact.

  N values in Soft Disintegrated Rock consistently exceed 50, showing refusal. No cores were obtained in SDR strata due to fissured nature of strata.
- c) Water table is encountered in the bore holes at 2.6 4.5 m.
- d) Open foundations are recommended resting in Soft Disintegrated Rock (SDR).
- e) SBC is recommended as 50 tonnes per sq m for foundations resting in Soft Disintegrated Rock (SDR). All foundations should be carried to SDR strata.
- f) This recommendation is based on the assumption of footings of width 2 m resting in SDR at 1.5 m depth below sub-cellar floor level. The actual size of the footings will be based on loads from the superstructure.



- g) All loose boulders and loose soil should be fully removed before placement of PCC bed.
- h) Since water table exists between 2.6 4.5 m depth, the foundation units need to be designed for uplift pressure. Further, adequate water proofing should be made.
- i) Angle of friction for SDR strata is recommended as 40 degrees.

(Dr. D. BABU RAO)

M.E., Ph.D. (USA), MIGS

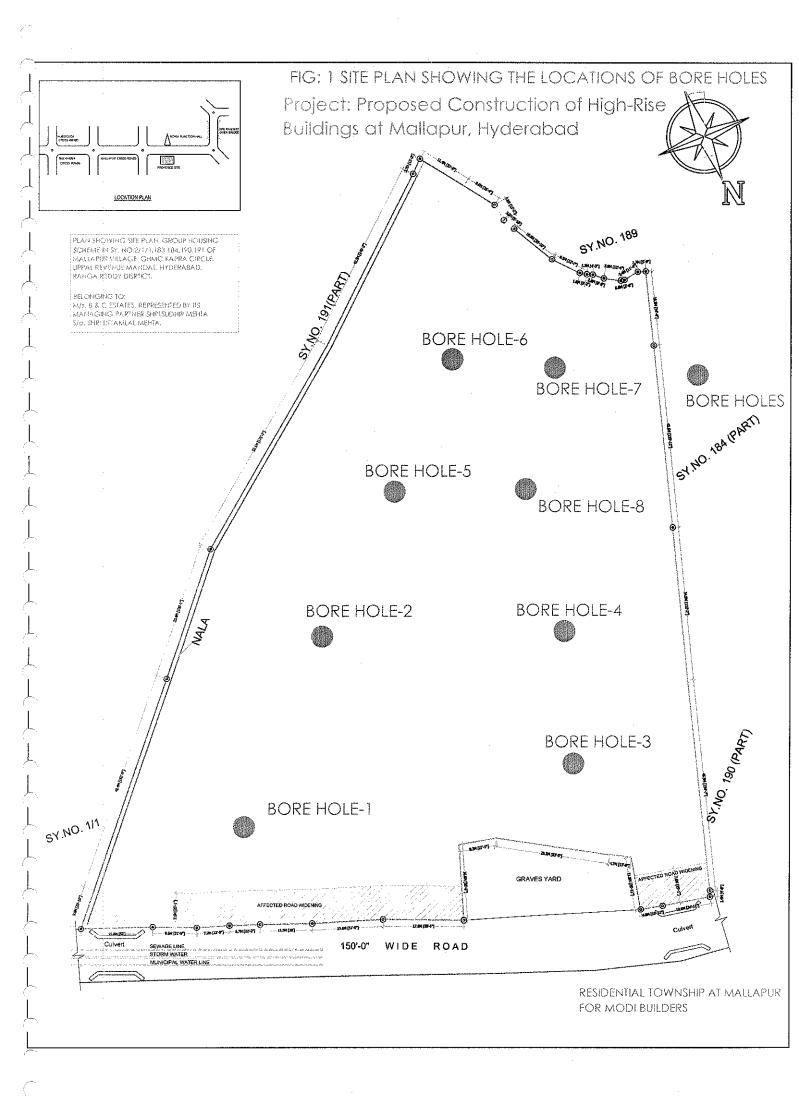
Former Professor & Head of Civil Engineering Principal Geotechnical Consultant

For **GEO TECHNOLOGIES** 

(Dr. N. VENKAT RAO)

M.Sc. Tech., Ph.D. FAEG, MIGS

Former Professor & Head of Geophysics Geological Consultant & Proprietor



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## FIG:2 LOG OF BORE HOLES

Project: Proposed Construction of Migh-Rise Buildings at Mallapur, Hyderabad



TABLE - 1

### SUMMARY OF DRILLING

Project : Proposed High-rise Building at Mallapur, Hyderabad.

S.No.	BORE HOLE No.	DRILLED DEPTH (m)	WATER TABLE DEPTH (m)
1	BH-1	6.50	4.50
2	BH-2	10.00	4.50
3	BH-3	10.00	4.10
4	BH-4	10.00	3.80
5	BH-5	10.00	3.50
6	BH-6	10.00	3.50
7	BH-7	10.00	2.60
8	BH-8	10.00	2.70

TABLE - 2

### SUMMARY OF SDR PROPERTIES

Project: Proposed High-rise Building at Mallapur, Hyderabad.

DU	Cample	Specific	w.c.	γ	Shear F	Parameters
BH No.	Sample Depth, m	Gravity	%	kN/cu m	С	φ
140.	Deptii, iii				kN/m2	deg
BH-2	8.00	2.70	7.5	19.3	0	38
DI 1-2	9.00	2.69	7.1	19.5	0	40
	8.00	2.70	6.8	19.5	0	42
BH-3	9.00	2.72	6.5	19.8	0	45
	10.00	2.72	6.2	20.0	0	45
	6.00	2.67	8.5	19.2	0	40
BH-4	7.50	2.68	7.9	19.4	0	42
	9.00	2.68	7.4	19.5	0	45
BH-5	8.00	2,69	9.2	19.3	0	39
ט-ווט	9.00	2.70	8.7	19.6	0	43
BH-6	8.00	2.69	7.5	19.1	0	41
B11-0	9.00	2.72	6.8	19.5	0	42
BH-7	8.00	2.73	6.5	19.8	0	43
יוים	9.00	2.69	7.1	20.1	0	45
BH-8	8.00	2.71	6.6	19.6	0	42
בווים	9.00	2.73	5.9	19.9	0	44

### **NOTATION**

γ: Unit Weight (Natural Density)

C : Cohesion

w.c:Natural Moisture (Water) Content φ: Angle of Internal Friction

TABLE - 3

### **ROCK CHARACTERISTICS**

Project: Proposed High-rise Building at Mallapur, Hyderabad.

Rock : Granite

		<del></del>
1	TEXTURE	Coarse Grained
2	STRUCTURE	Compact
3	HARDNESS	Hard
4	STREAK	Grey
5	REACTION WITH CHEMICALS	No reaction with acids
6	BEDDING	Not seen
7	JOINTS	Not seen
8	DIP	No discontinuities in the samples
9	STRIKE	Not observed in the samples
10	FAULTS	None by examination of samples

TABLE - 4

### RESULTS OF TESTS ON ROCK SAMPLES

Project: Proposed High-rise Building at Mallapur, Hyderabad.

Rock Granite

BORE HOLE No.	DEPTH OF SAMPLE (m)	SPECIFIC GRAVITY	POROSITY (%)	WATER ABSORPTION (%)	UCS kg/ sq cm
	3.50 - 4.50	2.71	2.80	3.40	860
BH-1	4.5 - 5.5	2.70	3.20	3.10	620
	5.5 - 6.5	2.74	2.40	2.70	950

Note: 1. All Tests are conducted in accordance with IS: 1124-974; IS: 9143-979.

2. When the length of the core is < 10 cm, UCS test is not conducted.

### APPENDIX

### CALCULATION OF SAFE BEARING CAPACITY

Project: Proposed High-rise Building at Mallapur, Hyderabad.

### Foundations in Soft Disintegrated Rock (SDR):

### I. Based on 'N' Values

### a) Shear criterion:

Assume width of Footings B as 2 m.

Assume corrected average value of 35 for N,

Assume D the depth of foundations as 1.5 m below cellar floor level.

Allowable bearing pressure is:

q (Allowable.) = 1/18 [2 x N x N B Rd + 6 ( 100 + N x N ) D Rw ]

= 875 kN / sq m

### b) Settlement Criterion:

Based on settlement criterion, for a permissible settlement of 40 mm,

Allowable bearing capacity = 12.3 N [(B + 0.3) / B] Rd Rw

 $= 495 \, \text{KN/sq m}$ 

### II. Based on IS: 12070

In accordance with IS: 12070, Soft Disintegrated Rock with 0% RQD is classified as **'very poor' (group-V)** and the recommended net bearing pressure as per the Code is 40 - 45 - 55 t / sq m.

Recommended Safe Bearing Capacity for foundations resting in Soft Disintegrated Rock (SDR) is 50 tonnes per sq m.

Annexure – 1:

Field Bore Log charts

							FIELD B	ORE L	FIELD BORE LOG CHART						111	
Projec	t: B&C	Estates	Project: B&C Estates Mallapur					8	Ш	Ground Level: Dia. Of Casing: NX	evel:					
Date	: 16-0.	7-2009 to	: 16-07-2009 to 17-07-2009						T O Z	Water Table: 4.50m	ble: 4.50	ı ı	į			
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From	Ç	(m)					(15-30-45)cm	Value	Pieces(cm)	Length (cm)	Pieces	Recovery	//0	<i>a),</i>		
0.0	0.4	0.4														
0.4	1.5	1.1	Ŝ.		1.50	SPT	8-7-9	15								
1.5	3.0	1.5	1		3.00	SPT	16cm/50blows	>50	,						-	
3.0	3.5	0.5	Olity Wolden										<del>,</del>			
3.5	4.5	1.0							59+17+15=91	96	3no+1 sp	%96	91%		Milky	
4.5	5.5	1.0	Hard Rock						11+10+10=31	28	3no+9	%/8	31%		Milky	
5.5	6.5	1.0							28+53+16=97	26	3no oniy	%26	%26		Milky	
			SDR=Soft Disintigrated Rock	Disintigrate	ed Rock		cm/50= no.of blows	o.of blow	SJ.							

							FIELDE	30RE	FIELD BORE LOG CHART	 						
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1.2	1.5	0.3	5		1.50	SPT	3.4.4	ω							Muddy	
1.5	3.0	1.5	Clayey Sand		3.00	TdS	6-7-12	19							Muddy	
3.0	3.5	9.0													Muddy	
3.5	4.5	1.0	Shity IMORUM		4.50	SPT	29cm/50blows	>50							Muddy	
4.5	6.0	1.5			00.9	SPT	19cm/50blows	>50							Muddy	
0.9	7.5	1.5	ושות ואיסותווו	1	7.50	SPT	8cm/50blows	>50							Brownish	
7.5	9.0	1.5	0.00		8.00 9.00	D/S SPT	5cm/50blows	>50							Brownish	
0.6	10.0	1.0	Ś		9.50	S/Q									Brownish	
			SDR=Soft Disinfigrated Rock	sinfigrafe	ad Rock		-05/wo	cm/50= no of blows	SWOK	COHOS	sn=small nieces	U				

SDR≃Soft Disintigrated Rock

cm/50= no.of blows

sp=small pieces

							FIELD B	ORE I	FIELD BORE LOG CHART							
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5.5	6.0	0.5		17 m	6.00	SPT	16cm/50blows	>50							Yellow	
6.0	5.7	1.5	a			D/S SPT	8cm/50blows	>50							Yellow	
7.5	9.0	1.5			8.00 9.00	D/S SPT	5cm/50blows	>50							Yellow	
9.0	10.0	1.0			10.00	S/O									Yellow	
			SDR=Soft Disintigrated Rock	<b>Disintigrate</b>	ed Rock		cm/50= no.of blows	of blow	/S							

SUK=Soft Disintigrated Kock

CK CM/SU= no.of bi

							FIELD B	ORE L	FIELD BORE LOG CHART							
Projec	of: B&C	) Estates	Project: B&C Estates Mallapur					8	BORE HOLE	Ground Level: Dia, Of Casing: NX	evel:					
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å	Depth				Sampling	e in	ra La		De	Details of Rock core	ck core					
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0.0	0.	1.0													Muddy	
1.0	1.5	0.5	Filling		1.50	SPT	5-12-12	24							Muddy	
1.5	2.5	1.0							,						Muddy	
2.5	3.0	0.5			3.00	SPT	7-14-17	31							Muddy	
3.0	4.0	1.0													Brownish	
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7.5	0.6	1.5			8.00 9.00	D/S SPT	5cm/50blows	>50							Yellow	
9.0	10.0	1.0	and the latest and th		9.50	D/S									Yellow .	
			SDR=Soft Disintigrated Rock	isintigrate	ed Rock		cm/50= no.of blows	o.of blow	SI		1					

SDR=Soft Disintigrated Rock

cm/50= no.of blows

GEO TECHNOLOGIES

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Date	19-07-	: 19-07-2009 to 20-07-2009	3-07-2009					* [	>	Water T.	Water Table: 3.50m	m				
8	ž	encth			Sampling	9	25		De	ie ? ?	Details of Rock core			*		
500		' '5 \ \bar{2}	Description	Log Sore	Depth	Appe	No. blows for Penetration of	v.eg hen	>10cm core	<b>1</b>	No. of	No. of % of core		j 2 %	Water	
From	0	Philip ex ses ses ses			Jacobs, Side Side Side Side		(15-30-45)cm	Z E	Pieces(cm)		Pieces	Recovery				
0.0	1.0	1.0		19.00 19.00 19.00 19.00												I
1.0	1.5	0.5	Ô IIIII		1.50	SPT	4-5-5	10							Muddy	
1.5	3.0	1.5			3.00	SPT	6-7-12	6							Muddy	
3.0	4.0	1.0	Olliy Clay	5-6- 6-6- 6-6-		<u></u>									Muddy	
4.0	4.5	0.5	Silty Morney		4.50	SPT	28cm/50blows	>50							Muddy	
4.5	5.5	1.0		ing the real	5.50	S/O									Brownish	
5.5	6.0	0.5	Hard Morum		9.00	SPT	7cm/50blows	>50							Brownish	
6.0	2.5	1.5	and the second		6.50 7.50	D/S SPT	5cm/50blows	>50							Yellow	
7.5	9.0	1.5	SDR		8.00 9.00	D/S SPT	4cm/50blows	>50							Yellow	
9.0	10.0	1.0			9.50	S/O							***************************************		Yellow	
			5)	SDR=Soft Disintigrated Rock	Disintigra	ited Roc	نج				0	cm/50= no.of blows	Swol			

SUR-Soli Disintigrated Rock

GEO TECHNOLOGIES

							FELDB	ORE L	FIELD BORE LOG CHART							
Proje	ct: B&C	) Estates	Project: B&C Estates Mallapur					00	BOKE TOLE	Ground Level: Dia. Of Casing: NX	evel:					
Date	: 20-0	7-2009 to	Date : 20-07-2009 to 21-07-2009						٥ ٥	Water Table: 3.50m	ole: 3.50	ı				
å	Depth				Sampling	Ming	SPT		De	Details of Rock core	ck core					
		F o Z	Description	Log of Bore	Depth Type	Type	No. blows for Penetration of	<b>2</b>	>10cm core	Total	Š	No. of % of core	Sales s	\$ B 2	Water	A Second
Tom	ဝ	(m)			Ē		(15-30-45)ст	Value	Pieces(cm)	Length (cm)	Pieces	Recovery	7,0	7/6	,	
0.0	9'0	9.0													ı	ŀ
9.0	1.5	6.0	r ming		1.50	SPT	9-7-9	13							ļ	1
1.5	3.0	1.5	Silty Clay		3.00	SPT	10-24-22	46							1	Į
3.0	4.5	1.5													· .	
4.5	6.0	1.5	Sitty Wotalii		4.50	SPT	20cm/50blows	>50							1	
6.0	7.5	1.5	Hard Morum		6.00	SPT	12cm/50blows	>50							ļ	ı
7.5	8.0	0.5				D/S SPT	8cm/50blows	>50							_	
8.0	9.5	1.5	SDR		9.50	SPT	200/cm/50blows	>50								ļ
9.5	10.0	0.5													1	I
			SDR=Soft Disintigrated Rock	Disintigrate	3d Rock		cm/50= no.of blows	of blows								

							FIELD B	SORE!	FIELD BORE LOG CHART							
D roigi	÷ 080	10000 10000	Droicoft B&C Estatos Mallanus					٥	III CI III CI	Ground Level:	ivel:					
ပ် (၁)	יי ב	rorates.	Wallapui					3	1 2 2	Dia. Of Casing: NX	sing: NX					
Date	: 21-07	7-2009 tc	: 21-07-2009 to 22-07-2009						NC.	Water Table: 2,60m	le: 2.60r					
De	Depth		-		Sampling	E C	TAS		De	Details of Rock core	ck core					
ilean Sauger	Andrews Andrews Seese	Ē 5 Ē	Description	Log of Depth Type	E CO		No. blows for Penetration of	eliau	×10cm core	Ç	Š	% of core	m 0		Water	Age of large
T 5	To	(u)			(111)		(15-30-45)cm	Vaile	Pieces(cm)	Length (cm)	Pieces	Recovery	%	×.		production of the control of the con
0.0	1.0	1.0													Muddy	
1.0	1.5	0.5	D		1.50	SPT	2-9-9	13							Muddy	
1.5	3.0	1.5	Silty Morum		3.00	SPT	16-20-15	35	•						Muddy	
3.0	4.5	<del>ر</del> ئ ت	=======================================												Muddy	
4.5	0.9	1.5	Hard Morum		4.50	SPT DS	28cm/50blows	>50							Brownish	
0.9	5.2	1.5			6.00 7.50	SPT DS	20cm/50blows	>50							Yellow	
7.5	8.0	0.5	aCV		7.50	SPT	9cm/50blows	>50							Yellow	
8.0	9.0	1.0			8.00 9.00	D/S SPT	7cm/50blows	>50							Yellow	
9.0	10.0	1.0			9.50	S/O									Yellow	
			SDR=Soft Disintigrated Rock	Disintigrate	ed Rock		cm/50= no.of blows	3.of blows	S							

GEO TECHNOLOGIES

							FIELD B(	JRE LC	FIELD BORE LOG CHART							
Project	B&CE	Project: B&C Estates Mallapur	allapur					\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \		Ground Level: Dia. Of Casing	Ground Level: Dia. Of Casing: NX	×				
Date:	: 22-07-2009	2009						mini Bad	xo O Z	Water T	Water Table: 2.70m	mí				
Dept	,	0			Sampling	<b>5</b> 7	S)		<u>್</u> ಷ	Details of Rock core	lock core					
politica, Since Security	مداهم تنتم	E G	Description	Log of Bore	Dept	- AG	No. blows for Penetration of	Z	>f0cm core	Total	No. O	% of care	Sar %	\$ \$ \$ %	Water	P E E
F E	Ç	100a. 1501 1604 1603 1603					(15-30-45)ст	Value	Pieces(cm)	Length (cm)	Pieces	Recovery				
0.0	1.5	1.5	Filling		1.50	SPT	5-5-6	7.							Muddy	
1.5	3.0	1.5	Silfa, 8 d see		3.00	SPT	11-22-25	47							Muddy	
3.0	4.0	1.0	oiity iviolulii												Muddy	
4.0	4.5	0.5			4.50	SPT	27cm/50blows	>50							Muddy	
4.5	5.5	1.0			5.50	D/S			,						Brownish	
5.5	6.0	0.5			00.9	SPT	14cm/50blows	>50							Yellow	
6.0	7.5	<del>ل</del> ئ			7.50	SPT	8cm/50blows	>50							Yellow	
7.5	8.0	6.0	SDR		8.00	S/O									Yellow	
8.0	9.0	1.0		erege Strang Black	9.00	SPT	7cm/50blows	>50							Yellow	
0.6	10.0	1.0		en as Ve g	10.00	D/S									Yellow	
				SDR=Soft Disintigrated Rock	Disintigra	nted Roc	÷				0	cm/50= no.of blows	Swoj			

SUK=Soft Disintigrated Rock

### Annexure – 2

### BIS (IS) CODES

- 1. IS: 2131 1981: Method of Standard Penetration Test for Soils.
- 2. IS: 4968 (Part 1) 1976: Method for subsurface sounding for soils.
- 3. IS: 2132: Code of Practice for thin walled tube sampling of Soils.
- 4. IS: 2720 Part I onwards: Methods of Laboratory Tests for Soils.
- 5. IS: 1498 1970: Classification and Identification of Soils for General Engineering Purpose.
- 6. IS: 6403 1981: Code of Practice for determination of Bearing Capacity of Shallow Foundations.
- 7. IS: 12070 1987: Code of Practice for Design and Construction of Shallow Foundations on Rocks.
- 8. IS: 8009 1976 (Part I): Code of Practice for calculation of settlements of Foundations.
- 9. IS: 78 1983 Appendix I: Classification and Characteristics of Rocks.
- 10. IS: 1892 1962: Code of Practice for Site Investigations for Foundations.
- 11. IS: 4453 1985: Code of Practice for presentation of drilling information and core description in foundation investigation.
- 12. IS: 4078: Code of Practice for indexing and storage of drill cores.
- 13. IS: 6926 1996: Diamond Core Drilling for Site Investigation.
- 14. IRC: 78 2000: Standard Specifications and Code of Practice for Road Bridges section 7.