REPORT ON GEOTECHNICAL INVESTIGATION FOR PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENTS FOR M/S. MODI REALTY POCHARAM LLP AT POCHARAM, HYDERABAD



CLIENT: M/s. MODI REALTY POCHARAM LLP, Hyderabad

JANAURY - 2019

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1. INTRODUCTION

M/s. Modi Realty Pocharam LLP, Hyderabad has entrusted the work of carrying out Geotechnical Investigation for "Proposed Construction of Residential Apartments for M/s. Modi Realty Pocharam LLP at Pocharam, Hyderabad" to M/s. Avani Engineering Consultancy Pvt. Ltd., Hyderabad.

2. SCOPE OF WORK

The Scope of work includes the following

- Drilling of 3 No. boreholes at the proposed borehole locations as per the direction of the client.
- Excavation of 8 No. Trial Pits at the proposed locations as per the direction of the client.
- Collection of soil samples and carrying out the relevant laboratory tests on soil samples.
- Conducting Standard Penetration test (SPT) within the borehole at every 1.5m depth interval or as required.
- Preparation of Geotechnical investigation Report.

3. FIELD INVESTIGATIONS

3.1 Boreholes and Trial Pits

The fieldwork includes boring / drilling of 3 No of boreholes in soil and excavation of 8 No. Trial Pits. The details of 3 No. boreholes and 8 No. Trial Pits with respective depth of investigation are given in the table below.

List of Boreholes and Trial Pits with Depth of investigation

| SI. No. | Borehole No. / Trial Pit No. | Depth of Investigation (m) |
|------------|---------------------------------|-------------------------------|
| 1 | BH-1 | 15.0 |
| 2 | BH-2 | 15.0 |
| 3 | BH-3 | 15.0 |
| 4 | TP-1 | 9.0 |
| 5 | TP-2 | 9.0 |
| 6 | TP-3 | 9.0 |

| 7 | TP-4 | 9.0 |
|----|------|-----|
| 8 | TP-5 | 9.0 |
| 9 | TP-6 | 9.0 |
| 10 | TP-7 | 9.0 |
| 11 | TP-8 | 9.0 |

The location of boreholes is indicated in the layout plan and the same is given in Appendix-I.

3.2 Rotary drilling

Rotary drilling technique was adopted for advancing and cleaning out borehole in overburden soil between sample intervals. Standard penetration test (SPT) is conducted at every 1.5m interval or at change of strata.

3.3 Drilling equipment

Mechanical rigs of rotary drilling machine was used for the purpose. The rigs are well suited for the required work. The rigs are new and well-maintained.

Soil Boring: The boring was advanced by rotating drill string connected by series of drill rods. The boring diameter of 150mm was formed using a drag bit (Soil Cutter) in soil. The drag bit cuts the subsoil at the bottom of the borehole and the soil fragments were removed by drilling mud (bentonite) which is under circulation.

Bentonite slurry is used for stabilizing the borehole. Bentonite slurry deposits as a thin film on the wall of the borehole during circulation and stability of borehole is derived from its thixotropic property and the hydrostatic head of the suspension. Bentonite having liquid limit greater than 400% is used for preparing bentonite slurry.

Rock Drilling: The rock drilling was advanced by rotating drill string connected by series of drill rods. The formed diameter is of NX size using diamond bits connected through the double tube core barrel. The drill bit cuts the weathered / hard rock and the fragments of the cut materials were removed by circulating fresh water. Guide casing of size "NX" was used up to the weathered rock layer

3.4 Observation during drilling

Sampling

The samples collected from the SPT sampler are treated as disturbed samples. Immediately after collection of sample from the sampler, the sample was transferred to a double walled polythene bag. A sample label is attached and the bag is tied tightly with thread to avoid any loss of moisture.

Undisturbed Sample

Undisturbed Samples are collected in Clayey Stratum where ever possible.

Standard Penetration Test:

This test was performed as per IS 2131-1981. The split spoon sampler was lowered and driven under impact of a 63.5 kg load with a free fall of 75 cm for 45 cm penetration at the bottom of the hole. Initial 15 cm penetration is considered as seating drive and number of blows required to penetrate the sampler for remaining 30 cm (out of 45 cm mark usually driven in soil) is recorded as 'N' value at that depth.

If the sampler does not fully penetrate into soil, then the test is terminated after 50 blows and corresponding penetration is noted down.

Ground Water Table:

Observations were made for ground water in borehole during and after boring. Observed Ground water table is recorded on the borelog.

Preparation of Bore Logs:

On completion of each borehole, the soil samples were examined and logged. The final log is prepared on the basis of visual examination of soil samples and laboratory testing data. The following are observed and recorded in the borelog:

The commencement and completion date, location of borehole, elevation of top and bottom of boring, ground water table, boundaries of each soil layer that encountered, classification and description of the soil, blow count values obtained from SPT tests, collection of soil samples etc. Field borelogs are provided in Appendix-II.

4. LABORATORY TESTINGS

SOIL

Natural Moisture content

Natural moisture content of the soil samples is determined as per IS 2720 (Part 21) from the collected soil samples using oven-drying method.

Grain Size Distribution

Grain size distribution analysis was carried out by sieving method as per IS 2720 (Part 4) to determine the grain size distribution.

Atterberg's Limits

Atterberg's limits test was conducted as per IS: 2720 (Part 5) on the soil samples collected to determine the liquid limit and plastic limit.

Shear Test

Shear Test is conducted as per IS: 2720 (Part 11 / 13).

Free Swell Index

Free Swell index of soil is determined as per IS: 2720 (Part 40).

Laboratory results of soil are provided in Appendix - III.

5. SUB SURFACE PROFILE

Based on the field and laboratory test results, the general subsurface profiles encountered in the boreholes are has given below.

| Borehole No. / | Depth (m) | | Thickness of Layer | Description of the Soil | IS Classifi | |
|-------------------|--------------|--|--------------------|--|----------------|--|
| Trial Pit No. | From | То | (m) | Description of the 3011 | cation | |
| | 0.0 | 3.0 | 3.0 | Reddish Brown Clayey Sand (Murrum) | | |
| BH-1 | 3.0 | 6.0 | 3.0 | Brownish Grade VI Residual Soil | SM / SP | |
| БП-1 | 6.0 | 9.0 | 3.0 | Completely Weathered Rock to Weathered Rock | - | |
| | 9.0 15.0 6.0 | Weathered Rock to Hard Rock (Granite) | - | | | |

| | 0.0 | 3.0 | 3.0 | Reddish Brown Clayey Sand (Murrum) | SC |
|--------|-----|------|-----|---|---------|
| BH-2 | 3.0 | 6.0 | 3.0 | Brownish Grade VI Residual Soil | SM / SP |
| DI 1-2 | 6.0 | 9.0 | 3.0 | Completely Weathered Rock to Weathered Rock | - |
| | 9.0 | 15.0 | 6.0 | Weathered Rock to Hard Rock (Granite) | - |
| | 0.0 | 3.0 | 3.0 | Reddish Brown Clayey Sand (Murrum) | SC |
| | 3.0 | 6.0 | 3.0 | Brownish Grade VI Residual Soil | SM / SP |
| BH-3 | 6.0 | 9.0 | 3.0 | Completely Weathered Rock to Weathered Rock | - |
| | 9.0 | 15.0 | 6.0 | Weathered Rock to Hard Rock (Granite) | - |
| | 0.0 | 3.0 | 3.0 | Reddish Brown Clayey Sand (Murrum) | - |
| TP-1 | 3.0 | 6.0 | 3.0 | Brownish Grade VI Residual Soil | - |
| | 6.0 | 9.0 | 3.0 | Completely Weathered Rock to Weathered Rock | - |
| | 0.0 | 3.0 | 3.0 | Reddish Brown Clayey Sand (Murrum) | - |
| TP-2 | 3.0 | 6.0 | 3.0 | Brownish Grade VI Residual Soil | - |
| | 6.0 | 9.0 | 3.0 | Completely Weathered Rock to Weathered Rock | - |
| | 0.0 | 3.0 | 3.0 | Reddish Brown Clayey Sand (Murrum) | - |
| TP-3 | 3.0 | 6.0 | 3.0 | Brownish Grade VI Residual Soil | - |
| | 6.0 | 9.0 | 3.0 | Completely Weathered Rock to Weathered Rock | - |
| | 0.0 | 3.0 | 3.0 | Reddish Brown Clayey Sand (Murrum) | - |
| TP-4 | 3.0 | 6.0 | 3.0 | Brownish Grade VI Residual Soil | - |
| | 6.0 | 9.0 | 3.0 | Completely Weathered Rock to Weathered Rock | - |
| TD 5 | 0.0 | 3.0 | 3.0 | Reddish Brown Clayey Sand (Murrum) | - |
| TP-5 | 3.0 | 6.0 | 3.0 | Brownish Grade VI Residual Soil | - |

| | 6.0 | 9.0 | 3.0 | Completely Weathered Rock to Weathered Rock | - |
|------|-----|-----|-----|--|---|
| | 0.0 | 3.0 | 3.0 | Reddish Brown Clayey Sand (Murrum) | - |
| TP-6 | 3.0 | 6.0 | 3.0 | Brownish Grade VI Residual Soil | - |
| | 6.0 | 9.0 | 3.0 | Completely Weathered Rock to Weathered Rock | - |
| | 0.0 | 3.0 | 3.0 | Reddish Brown Clayey Sand (Murrum) | - |
| TP-7 | 3.0 | 6.0 | 3.0 | Brownish Grade VI Residual Soil | - |
| | 6.0 | 9.0 | 3.0 | Completely Weathered Rock to Weathered Rock | - |
| | 0.0 | 3.0 | 3.0 | Reddish Brown Clayey Sand (Murrum) | - |
| TP-8 | 3.0 | 6.0 | 3.0 | Brownish Grade VI Residual Soil | - |
| | 6.0 | 9.0 | 3.0 | Completely Weathered Rock to Weathered Rock | - |

The sub-surface profile is shown in Appendix-IV.

6. TYPE OF FOUNDATION

Based on the sub-surface profile and the type of structure i.e., Residential Apartments envisaged, Open foundation can be considered.

7. SHALLOW FOUNDATION

The following section describes the method of analysis adopted to arrive at the safe bearing capacities for shallow foundation.

Bearing capacity calculation for footings resting on soil / weathered rock

In case of sandy soils, the angle of internal friction 'Ø' is determined by the standard penetration test as per IS: 6403. The Ø values were used for determining the Ultimate Bearing Capacity of soil as per shear failure criteria. The ultimate bearing capacity, qd is calculated as per equations furnished in IS: 6403 as given below.

The ultimate bearing capacity q_d is given by

$$q_{nu} = c N_c s_c d_c i_c + q (N_q - 1) s_q d_q i_q + 1/2 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma} w'$$

q_{nu} = Net ultimate bearing capacity

c = cohesion

 ϕ = Angle of internal friction

 γ = Unit weight of soil

q = Over burden pressure = depth x density = d x γ

B = Width of the foundation

W' = Water table correction factor

 N_c , N_q , N_γ = Bearing capacity factors

 s_c , s_q , s_γ = Shape factors

For strip footing $s_c = s_q = s_y = 1$

For rectangular footing $s_c = 1 + 0.2 \times B / L$,

 $s_q = 1 + 0.2 \times B / L$

 $s_{\gamma} = 1 - 0.4 \times B / L$

For square footing $s_c = 1.3$, $s_q = 1.2$, $s_\gamma = 0.8$

For circular footing $s_c = 1.3$, $s_q = 1.2$, $s_\gamma = 0.6$

 d_c , d_q , d_v = Depth factors

 $d_c = 1 + 0.2 \times (D_f / B) \tan (45^\circ + \phi' / 2)$

 $d_{\alpha} = d_{\gamma} = 1$ for $\phi' < 10^{\circ}$

 $d_q = d_\gamma = 1 + 0.1 \text{ x } (D_f / B) \tan (45^\circ + \phi' / 2) \text{ for } \phi' > 10^\circ$

 i_c , i_a , i_v = Inclination factors

$$i_c = i_a = (1 - \alpha^{\circ} / 90^{\circ})^2$$

$$i_v = (1 - \alpha^{\circ} / \phi^{\circ})^2$$

 α° = Inclination of the load with vertical

The Safe bearing capacity (SBC) is the maximum intensity of loading that the foundation will safely carry without the risk of shear failure of soil. Safe bearing capacity can be obtained by dividing the Ultimate Bearing Capacity (UBC) with a factor of safety of 2.5.

$$SBC = q_{nu} / 2.5$$

In the design of bearing capacity calculations the water table is assumed at foundation level and water table correction factor has been applied accordingly.

The computation of bearing capacity calculation is provided in Table - 1 to 3 of Appendix-V.

8. RECOMMENDATION

- Based on the sub-surface profile encountered for construction of Residential Apartments, Shallow Foundation can be considered.
- The proposed Residential Apartments has 2 cellars. The total depth of excavation required for 2 cellars is 6.0m to 7.5m. Excavated soil from the cellar will be of the nature of murrum, residual soil, completely weathered rock and weathered rock.
- The recommended net safe bearing capacity along with founding depth for borehole locations drilled is given in the table below.

| Borehole No. | Depth of Foundation below the Bottom of Cellar Level (m) | Recommended Net Safe Bearing Capacity (t/sqm) | Foundation Stratum |
|-----------------|--|---|---------------------------|
| | 1.5 | 65.0 | Completely Weathered Rock |
| BH-1 to BH-3 | 2.0 | 75.0 | Completely Weathered Rock |
| and TP-1 to | 2.5 | 85.0 | Completely Weathered Rock |
| TP-8 | 1.0 | 70.0 | Weathered Rock |
| | 1.5 | 80.0 | Weathered Rock |

- In general the foundation stratum at a depth of 6.0m to 9.0m below the existing ground is of the nature of Completely Weathered Rock / Weathered Rock.
- The foundation stratum at a depth of 9.0m to 12.0m below the existing ground is of the nature of Weathered Rock / Hard Rock.
- Each foundation at the time of construction shall be judged suitable for the type of stratum i.e., Completely Weathered Rock / Weathered Rock and bearing capacity taken accordingly.
- Any variation in the foundation stratum shall be brought to the notice of the Geotechnical Engineer.

G. SATEESH KUMAR Geotechnical Engineer

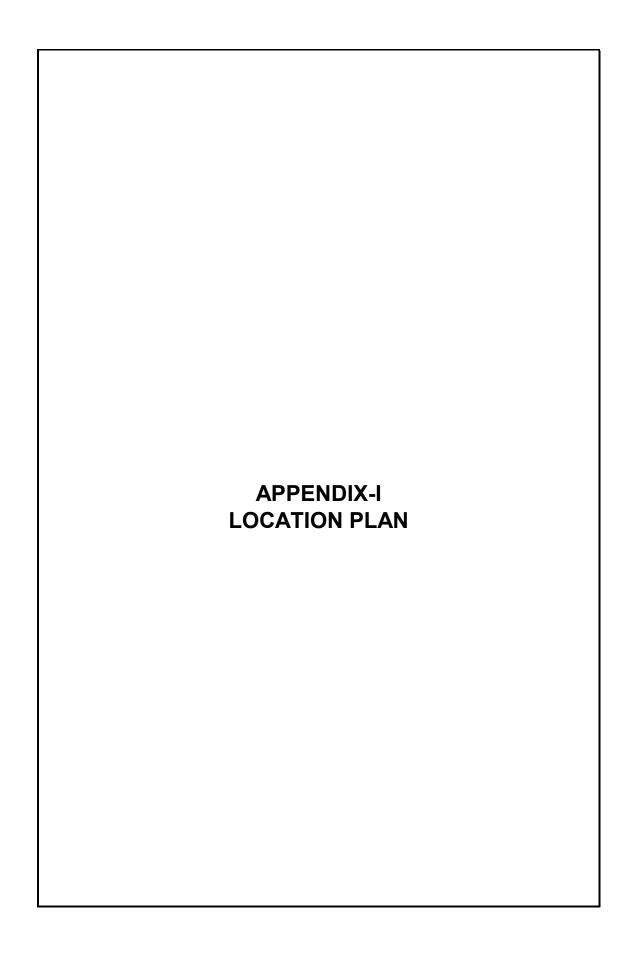
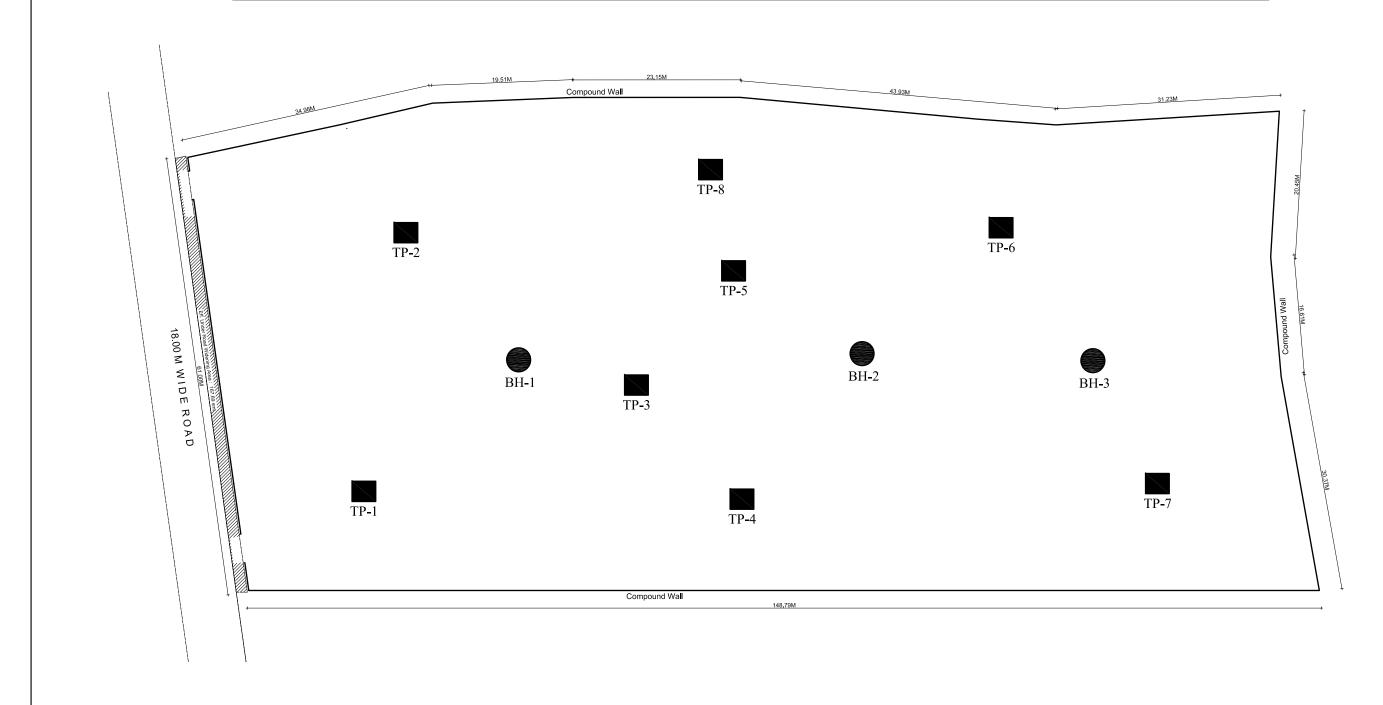


FIGURE-1: LOCATION PLAN SHOWING BOREHOLES AND TRIAL PITS



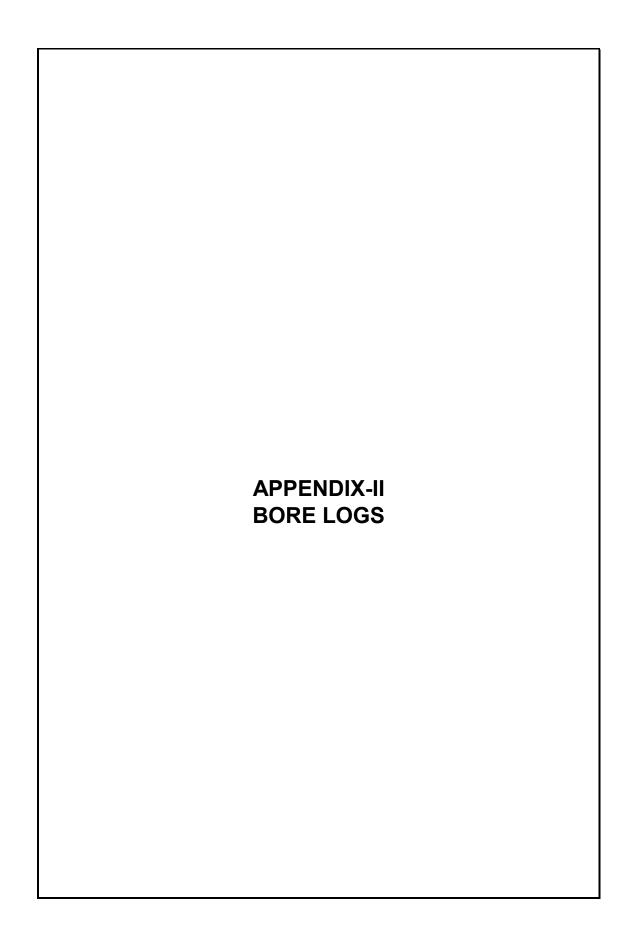


TABLE-1: BORELOG

| Boring method : Rotary with mud circulation | | Record of BH-1 | | BH-1 | | |
|---|--------------|------------------------------------|---|----------------|------------|--|
| | | Water level: Not | Date Commenced : | 20.01.2019 | | |
| oring equipment: Caly | x | encountered upto the drilled depth | Date Completed : | 21.01.2019 | | |
| Samples and In-situ test | | | Description of Strata | Depth From EGL | Soil Group | |
| Depth (m) | Type & Value | | Description of Ottata | Depart for EGE | Goil Group | |
| EGL | | | | | | |
| | | Reddish | Reddish Brown Clayey Sand (Murrum) | | | |
| 1.5 | DS | rteddisi | T Brown Glayby Gana (Manani) | | | |
| | | | | | SC | |
| 3.0 | DS | | | | | |
| | | | | | | |
| 4.5 | DS | Brow | nish Grade VI Residual Soil | | | |
| | | | | | | |
| 6.0 | DS | | | | | |
| | | | | 6.0 | SM / SP | |
| 6.0 | 0 | | | | | |
| to | Core | Con | Completely Weathered Rock to Weathered Rock | | | |
| 9.0 | | | | | | |
| 9.0 | | | | 9.0 | - | |
| to 9.0 | Core | | | | | |
| 12.0 | Core | | | | | |
| 12.0 | | | | | | |
| 12.0 | | Weather | red Rock to Hard Rock (Granite) | | | |
| to | Core | | | | | |
| 15.0 | 33.3 | | | | | |
| | | | | 15.0 | _ | |
| | | | | 10.0 | | |
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TABLE-2: BORELOG

| Boring method : Rotary with mud circulation | | Record of BH-2 | Record of BH-2 | | BH-2 | | |
|---|-----------------------|------------------------------------|---|----------------|------------|--|--|
| Poring oguir-rate Oat | | Water level: Not | Date Commenced : | 22.01.2019 | | | |
| Boring equipment: Caly | yx | encountered upto the drilled depth | Date Completed : | 23.01.2019 | | | |
| Samp | oles and In-situ test | | Description of Strata | Depth From EGL | Soil Group | | |
| Depth (m) | Type & Value | | Description of ottata | Dopurrionico | Ooii Group | | |
| EGL | | | | | | | |
| 1.5 | DS | Reddish | Reddish Brown Clayey Sand (Murrum) | | | | |
| 3.0 | DS | | | | SC | | |
| 4.5 | DS | Brow | Brownish Grade VI Residual Soil | | | | |
| 6.0 | DS | | | | | | |
| 6.0 | | | | 6.0 | SM / SP | | |
| to | Core | | onletely Weathers & Dealete | | | | |
| 9.0 | Oule | Con | Completely Weathered Rock to Weathered Rock | | | | |
| 5.0 | | | | 9.0 | | | |
| 9.0 | | | | 9.0 | - | | |
| to | Core | | | | | | |
| 12.0 | | | | | | | |
| | | | | | | | |
| 12.0 | | Weather | ed Rock to Hard Rock (Granite) | | | | |
| to | Core | | | | | | |
| 15.0 | | | | | | | |
| | | | | 15.0 | _ | | |
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TABLE-3: BORELOG

| Boring method : Rotary with mud circulation | | Record of BH-3 | Record of BH-3 | | BH-3 | |
|---|------------------------|------------------------------------|--|----------------|------------|--|
| Poring og viras et 4 | 20hay | Water level: Not | Date Commenced : | 24.01.2019 | | |
| Boring equipment: (| -aiyx | encountered upto the drilled depth | Date Completed : | 25.01.2019 | | |
| Sa | mples and In-situ test | | Description of Strata | | Soil Group | |
| Depth (m) | Type & Value | | Boompastron Guata | Depth From EGL | | |
| EGL | | | | | | |
| 1.5 | DS | Reddish | Reddish Brown Clayey Sand (Murrum) | | | |
| 3.0 | DS | | | 3.0 | SC | |
| 4.5 | DS | Brow | rnish Grade VI Residual Soil | | | |
| 6.0 | DS | | | 6.0 | SM / SP | |
| 6.0 | | | | 0.0 | SIVI / SF | |
| to 9.0 | Core | Com | Completely Weathered Rock to Weathered Rock | | | |
| 0.0 | | | | | _ | |
| 9.0 | | | | 9.0 | | |
| to | Core | | | | | |
| 12.0 | | | | | | |
| 40.0 | | Weather | ed Rock to Hard Rock (Granite) | | | |
| 12.0 to | Core | | | | | |
| 15.0 | Core | | | | | |
| 10.0 | | | | 15.0 | _ | |
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TABLE-4: TRIAL PIT LOG

| Method - Trial Pit Excavation by JCB | | Record of TP-1 | | TP-1 | |
|--------------------------------------|--------------------------------|--------------------------------------|--------------------------|----------------|------------|
| | | Water level. Not encountered upto | | 22.01.2019 | |
| | | the excavated depth Date Completed : | | 22.01.2019 | |
| Samples a | and In-situ test Type & Value | Descrip | otion of Strata | Depth From EGL | Soil Group |
| EGL | | | | | |
| 1.5 | DS | Reddish Brown C | Clayey Sand (Murrum) | 0.0 | |
| 3.0 | DS | | | 3.0 | - |
| 4.5 | DS | Brownish Grad | de VI Residual Soil | | |
| | | | | 6.0 | - |
| 6.0 | DS | | | | |
| 7.5 | DS | Completely Weathered | d Rock to Weathered Rock | | |
| 9.0 | DS | | | 9.0 | - |
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TABLE-5: TRIAL PIT LOG

| Method - Trial Pit Excavation by JCB | | Record of TP-2 | TP-2 | TP-2 | | |
|--|----|--------------------------------------|---|----------------|------------|--|
| | | Water level. Not encountered upto | | 22.01.2019 | 22.01.2019 | |
| | | the excavated depth Date Completed : | | 22.01.2019 | 22.01.2019 | |
| Samples and In-situ test Depth (m) Type & Value | | - Descri _l | otion of Strata | Depth From EGL | Soil Group | |
| EGL | | | | | | |
| 1.5 | DS | Reddish Brown C | Clayey Sand (Murrum) | | | |
| 3.0 | DS | | | 3.0 | - | |
| 4.5 | DS | Brownish Gra | de VI Residual Soil | | | |
| 6.0 | DS | | | 6.0 | - | |
| 7.5 | DS | Completely Weathere | Completely Weathered Rock to Weathered Rock | | | |
| 9.0 | DS | | | | - | |
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TABLE-6: TRIAL PIT LOG

| ethod - Trial Pit Exc | cavation by JCB | Record of TP-3 | TP-3 |
|-----------------------|------------------|--|--------------------------|
| | | Water level: Not encountered upto Date Commenced : | 22.01.2019 |
| | | the excavated depth Date Completed : | 22.01.2019 |
| | and In-situ test | Description of Strata | Depth From EGL Soil Grou |
| Depth (m) | Type & Value | | |
| LGL | | | |
| 1.5 | DS | Reddish Brown Clayey Sand (Murrum) | |
| 3.0 | DS | | 3.0 - |
| 4.5 | DS | Brownish Grade VI Residual Soil | |
| 6.0 | DS | | 6.0 - |
| 7.5 | DS | Completely Weathered Rock to Weathered Roc | k |
| 9.0 | DS | | 9.0 |
| | | | 9.0 - |
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TABLE-7: TRIAL PIT LOG

| /lethod - Trial Pit Ex | cavation by JCB | Record of TP-4 | | TP-4 | |
|------------------------|--------------------------------|-----------------------------------|--------------------------|----------------|------------|
| | | Water level: Not encountered upto | Date Commenced : | 22.01.2019 | |
| | | the excavated depth | Date Completed : | 22.01.2019 | |
| Samples a | and In-situ test Type & Value | - Descri _l | otion of Strata | Depth From EGL | Soil Group |
| EGL | | | | | |
| 1.5 | DS | Reddish Brown C | Clayey Sand (Murrum) | | |
| 3.0 | DS | | | 3.0 | - |
| 4.5 | DS | Brownish Gra | de VI Residual Soil | 0.0 | |
| 6.0 | DS | | | 6.0 | - |
| 7.5 | DS | Completely Weathere | d Rock to Weathered Rock | | |
| 9.0 | DS | | | 9.0 | - |
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TABLE-8: TRIAL PIT LOG

| ethod - Trial Pit Exc | cavation by JCB | Record of TP-5 | | TP-5 | |
|-----------------------|------------------|-------------------------------------|----------------------|----------------|------------|
| | | vvalor level. Not enlocationed apto | Commenced : | 22.01.2019 | |
| | | the excavated depth Date | Completed : | 22.01.2019 | |
| | and In-situ test | - Description o | f Strata | Depth From EGL | Soil Group |
| Depth (m) | Type & Value | | | | |
| EGL | | | | | |
| 1.5 | DS | Reddish Brown Claye | y Sand (Murrum) | | |
| 3.0 | DS | | | 3.0 | - |
| 4.5 | DS | Brownish Grade V | l Residual Soil | | |
| 6.0 | DS | | | 6.0 | - |
| 7.5 | DS | Completely Weathered Ro | ck to Weathered Rock | | |
| 9.0 | DS | | 9.0 | _ | |
| | | | | 9.0 | - |
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TABLE-9: TRIAL PIT LOG

| lethod - Trial Pit Exc | cavation by JCB | Record of TP-6 | | TP-6 | |
|------------------------|--------------------------------|-----------------------------------|--------------------------|----------------|--------------|
| | | Water level: Not encountered upto | Date Commenced : | 22.01.2019 | |
| | | the excavated depth | Date Completed : | 22.01.2019 | |
| Samples a | and In-situ test Type & Value | - Descri _l | otion of Strata | Depth From EGL | Soil Group |
| EGL | | | | | |
| 1.5 | DS | Reddish Brown C | Clayey Sand (Murrum) | | |
| 3.0 | DS | | | 3.0 | - |
| 4.5 | DS | Brownish Gra | de VI Residual Soil | | |
| 6.0 | DS | | | 6.0 | - |
| 7.5 | DS | Completely Weathere | d Rock to Weathered Rock | | |
| 9.0 | DS | | | 9.0 | - |
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TABLE-10: TRIAL PIT LOG

| lethod - Trial Pit Exc | cavation by JCB | Record of TP-7 | | TP-7 | |
|------------------------|--------------------------------|-----------------------------------|--------------------------|----------------|------------|
| | | Water level: Not encountered upto | Date Commenced : | 22.01.2019 | |
| | | the excavated depth | Date Completed : | 22.01.2019 | |
| Samples a | and In-situ test Type & Value | - Descri _l | otion of Strata | Depth From EGL | Soil Group |
| EGL | | | | | |
| 1.5 | DS | Reddish Brown C | Clayey Sand (Murrum) | | |
| 3.0 | DS | | | 3.0 | <u>-</u> |
| 4.5 | DS | Brownish Grad | de VI Residual Soil | 0.0 | |
| 6.0 | DS | | | 6.0 | - |
| 7.5 | DS | Completely Weathere | d Rock to Weathered Rock | | |
| 9.0 | DS | | | 9.0 | - |
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TABLE-11: TRIAL PIT LOG

| lethod - Trial Pit Exc | cavation by JCB | Record of TP-8 | | TP-8 | |
|------------------------|--------------------------------|-----------------------------------|--------------------------|----------------|------------|
| | | Water level: Not encountered upto | Date Commenced : | 22.01.2019 | |
| | | the excavated depth | Date Completed : | 22.01.2019 | |
| Samples a | and In-situ test Type & Value | - Descri _l | otion of Strata | Depth From EGL | Soil Group |
| EGL | | | | | |
| 1.5 | DS | Reddish Brown C | Clayey Sand (Murrum) | | |
| 3.0 | DS | | | 3.0 | - |
| 4.5 | DS | Brownish Gra | de VI Residual Soil | | |
| 6.0 | DS | | | 6.0 | - |
| 7.5 | DS | Completely Weathere | d Rock to Weathered Rock | | |
| 9.0 | DS | | | 9.0 | - |
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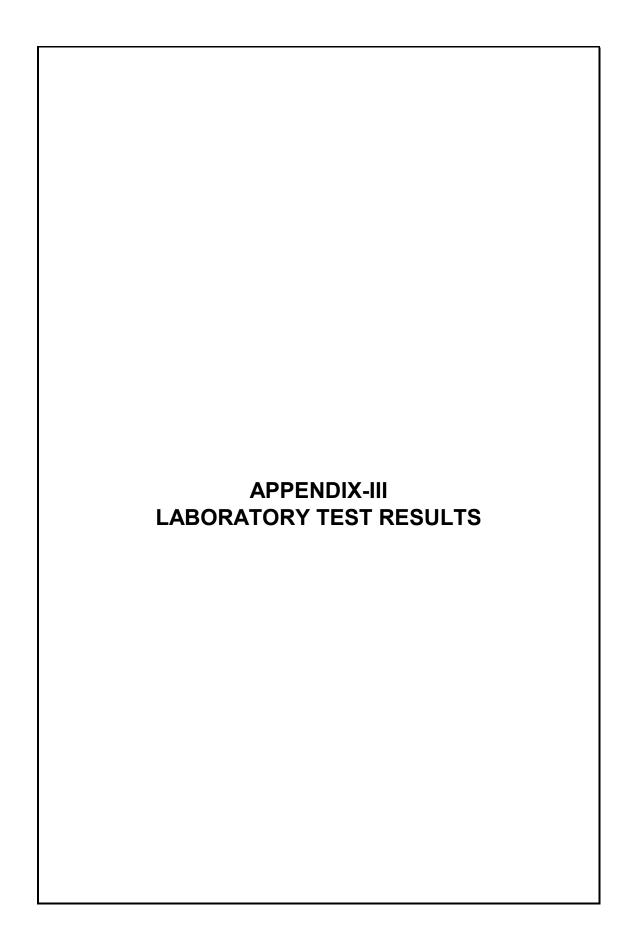


TABLE-1: SUMMARY OF LABORATORY TEST RESULTS OF SOIL SAMPLES

PROJECT: GEOTECHNICAL INVESTIGATION FOR PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENTS FOR M/S. MODI REALTY POCHARAM LLP AT POCHARAM, HYDERABAD

| | | | Т | Т | | | 1 | 1 | | | | | | , | | | | | |
|--------------------------|-----------|--------|----------------|----------------------|---------------------|----------------------|------------------|---------------|---------------|---------------|-------------|-----------------------|-------------------|-----------------------|---------------------|--------------------|--------------|-------------------------------|-----------------------------------|
| 9 | | | | (% | | | | | GRAIN SIZ | E DISTRIBL | JTION | | | | | | | SHEAR TE | ST |
| TIT 1 | | | Щ | , F | | _ | Ä | | | SAND | | | NO NO | X | (၁၁/f | (cc) | | PT | 7 |
| BOREHOLE / TRAIL PIT NO. | DEРТН (m) | SPT/UD | TYPE OF SAMPLE | MOISTURE CONTENT (%) | LIQUID LIMIT (%) | PLASTIC LIMIT (%) | PLASTICITY INDEX | GRAVEL (%) | COARSE (%) | MEDIUM (%) | FINE (%) | SILT & CLAY (%) | IS CLASSIFICATION | SWELLING INDEX (%) | BULK DENSITY (g/cc) | DRY DENSITY (g/cc) | TYPE OF TEST | COHESION INTERCEPT (t/sqm) | ANGLE OF INTERNAL FRICTION (°) |
| | Test M | ethod | | IS 2720 (Part 2) | IS | 2720 (Part | t 5) | | IS 2 | 720 (Part 4) | | | IS: 1498 | IS 2720 (Part 40) | SP 36 | (Part 2) | IS | 6 2720 (Part 11 | / 13) |
| BH-1 | 1.5 | 1 | DS | 12.5 | 36 | 17 | 19 | 3.5 | 21.7 | 32.5 | 18.3 | 24.0 | SC | - | - | - | - | - | - |
| BH-1 | 3.0 | - | DS | 10.3 | NP | NP | NP | 13.7 | 17.3 | 41.2 | 14.2 | 13.6 | SM | - | - | - | - | - | - |
| BH-1 | 4.5 | - | DS | 9.7 | NP | NP | NP | 18.9 | 28.9 | 37.3 | 12.7 | 2.2 | SP | - | - | - | - | - | - |
| BH-1 | 6.0 | - | DS | | | | | | C | ompletely W | eathered | Rock to | Weathered | d Rock | | | | | |
| BH-1 | 7.5 | - | DS | | | | | | C | ompletely W | eathered | Rock to | Weathered | d Rock | | | | | |
| BH-1 | 9.0 | - | DS | | | | | | | Weathere | d Rock to | Hard Ro | ock (Grani | te) | | | | | |
| BH-1 | 10.5 | ı | DS | | | | | | | Weathere | d Rock to | Hard Ro | ock (Grani | te) | | | | | |
| BH-1 | 12.0 | - | DS | | | | | | | Weathere | d Rock to | Hard Ro | ock (Grani | te) | | | | | |
| BH-1 | 13.5 | - | DS | | | | | | | Weathere | d Rock to | Hard Ro | ock (Grani | te) | | | | | |
| BH-1 | 15.0 | ı | DS | | | | | | | Weathere | d Rock to | Hard Ro | ock (Grani | te) | | | | | |

TABLE-2: SUMMARY OF LABORATORY TEST RESULTS OF SOIL SAMPLES

PROJECT: GEOTECHNICAL INVESTIGATION FOR PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENTS FOR M/S. MODI REALTY POCHARAM LLP AT POCHARAM, HYDERABAD

| | 1 | | ı | 1 | 1 | 1 | | | | | | | 1 | , | | | | | |
|--------------------------|-----------|--------|----------------|----------------------|---------------------|----------------------|------------------|---------------|---------------|---------------|-------------|-----------------------|-------------------|-----------------------|---------------------|--------------------|--------------|-------------------------------|-----------------------------------|
| 9 | | | | (% | | | | | GRAIN SIZ | E DISTRIBL | JTION | | | | | | | SHEAR TE | ST |
| Ξ | | | Щ | Þ | | _ | Ä | | | SAND | | | ON | X | (၁၁/t | (၁၁/ | | PT | ۲ |
| BOREHOLE / TRAIL PIT NO. | DEРТН (m) | SPT/UD | TYPE OF SAMPLE | MOISTURE CONTENT (%) | LIQUID LIMIT (%) | PLASTIC LIMIT (%) | PLASTICITY INDEX | GRAVEL (%) | COARSE (%) | MEDIUM (%) | FINE (%) | SILT & CLAY (%) | IS CLASSIFICATION | SWELLING INDEX (%) | BULK DENSITY (g/cc) | DRY DENSITY (g/cc) | TYPE OF TEST | COHESION INTERCEPT (t/sqm) | ANGLE OF INTERNAL FRICTION (°) |
| | Test M | ethod | | IS 2720 (Part 2) | IS | 2720 (Part | : 5) | | IS 2 | 720 (Part 4) | | | IS: 1498 | IS 2720 (Part 40) | SP 36 | (Part 2) | IS | S 2720 (Part 11 | / 13) |
| BH-2 | 1.5 | ı | DS | 15.2 | 39 | 19 | 20 | 6.8 | 18.9 | 36.7 | 15.2 | 22.4 | SC | 1 | - | - | - | - | - |
| BH-2 | 3.0 | - | DS | 13.8 | NP | NP | NP | 15.9 | 21.2 | 37.8 | 12.4 | 12.7 | SM | - | - | - | - | - | - |
| BH-2 | 4.5 | - | DS | 10.1 | NP | NP | NP | 13.5 | 32.2 | 35.1 | 17.3 | 1.9 | SP | - | - | - | - | - | - |
| BH-2 | 6.0 | - | DS | | | | | | C | ompletely W | eathered | Rock to | Weathered | d Rock | | | | | |
| BH-2 | 7.5 | - | DS | | | | | | C | ompletely W | eathered | Rock to | Weathered | d Rock | | | | | |
| BH-2 | 9.0 | - | DS | | | | | | | Weathere | d Rock to | Hard Ro | ock (Granit | te) | | | | | |
| BH-2 | 10.5 | - | DS | | | | | | | Weathere | d Rock to | Hard Ro | ock (Granit | te) | | | | | |
| BH-2 | 12.0 | - | DS | | | | | | | Weathere | d Rock to | Hard Ro | ock (Granit | te) | | | | | |
| BH-2 | 13.5 | - | DS | | | | | | | Weathere | d Rock to | Hard Ro | ock (Granit | te) | | | | | |
| BH-2 | 15.0 | - | DS | | | | | | | Weathere | d Rock to | Hard Ro | ock (Granit | te) | | | | | |

TABLE-3: SUMMARY OF LABORATORY TEST RESULTS OF SOIL SAMPLES

PROJECT: GEOTECHNICAL INVESTIGATION FOR PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENTS FOR M/S. MODI REALTY POCHARAM LLP AT POCHARAM, HYDERABAD

| | | | | T | ı | | 1 | 1 | | | | | 1 | 1 | ı | | 1 | | |
|--------------------------|-----------|--------|----------------|----------------------|---------------------|-------------------|------------------|---------------|---------------|---------------|-------------|-----------------------|-------------------|-----------------------|---------------------|--------------------|--------------|-------------------------------|--|
| o O | | | | (% | | | | | GRAIN SIZ | E DISTRIBL | JTION | | | | | | | SHEAR TE | ST |
| 1 | | | Ш | Ę | | _ | Ä | | | SAND | | | N O | X | (၁၁/t | (00/ | | PT | Ľ |
| BOREHOLE / TRAIL PIT NO. | DEРТН (m) | SPT/UD | TYPE OF SAMPLE | MOISTURE CONTENT (%) | LIQUID LIMIT (%) | PLASTIC LIMIT (%) | PLASTICITY INDEX | GRAVEL (%) | COARSE (%) | MEDIUM (%) | FINE (%) | SILT & CLAY (%) | IS CLASSIFICATION | SWELLING INDEX (%) | BULK DENSITY (g/cc) | DRY DENSITY (g/cc) | TYPE OF TEST | COHESION INTERCEPT (t/sqm) | ANGLE OF INTERNAL FRICTION (⁰) |
| | Test M | ethod | | IS 2720 (Part 2) | IS | 2720 (Part | t 5) | | IS 2 | 720 (Part 4) | | | IS: 1498 | IS 2720 (Part 40) | SP 36 | (Part 2) | IS | S 2720 (Part 11 | / 13) |
| BH-3 | 1.5 | - | DS | 16.8 | 39 | 19 | 20 | 9.3 | 13.5 | 32.1 | 15.8 | 29.3 | sc | - | - | - | - | - | - |
| BH-3 | 3.0 | - | DS | 14.5 | NP | NP | NP | 11.5 | 18.9 | 38.2 | 10.5 | 20.9 | SM | - | - | - | - | - | - |
| BH-3 | 4.5 | - | DS | 12.2 | NP | NP | NP | 21.8 | 28.9 | 37.2 | 10.5 | 1.6 | SP | - | - | - | - | - | - |
| BH-3 | 6.0 | - | DS | | | | | | C | ompletely W | eathered | Rock to | Weathered | d Rock | | | | | |
| BH-3 | 7.5 | - | DS | | | | | | C | ompletely W | eathered | Rock to | Weathered | d Rock | | | | | |
| BH-3 | 9.0 | - | DS | | | | | | | Weathere | d Rock to | Hard Ro | ock (Grani | te) | | | | | |
| BH-3 | 10.5 | - | DS | | | | | | | Weathere | d Rock to | Hard Ro | ock (Grani | te) | | | | | |
| BH-3 | 12.0 | 1 | DS | | | | | | | Weathere | d Rock to | Hard Ro | ock (Grani | te) | | | | | |
| BH-3 | 13.5 | - | DS | | | | | | | Weathere | d Rock to | Hard Ro | ock (Grani | te) | | | | | |
| BH-3 | 15.0 | - | DS | | | | | | | Weathere | d Rock to | Hard Ro | ock (Grani | te) | | | | | |

TABLE-4: SUMMARY OF LABORATORY TEST RESULTS OF SOIL SAMPLES

PROJECT: GEOTECHNICAL INVESTIGATION FOR PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENTS FOR M/S. MODI REALTY POCHARAM LLP AT POCHARAM, HYDERABAD

| . 0 | | | | (%) | | | | | GRAIN SIZ | E DISTRIBU | JTION | | | | | | | SHEAR TE | ST |
|-------------------------|-----------|----------|-----|---------------------|---|-------------------|------------------|---------------|---------------|---------------|-------------|-----------------------|-------------------|-----------------------|----------------|--------------------|--------------|-------------------------------|--|
| PIT | | | ?LE |) TN | _ | l _⊑ | OEX | | | SAND | ı | | NO. | EX | (a/cc) | g/cc) | | EPT | AL |
| BOREHOLE / TRAIL PIT NO | DEРТН (m) | au / Tas | _ | MOISTURE CONTE | LIQUID LIMIT (%) | PLASTIC LIMIT (%) | PLASTICITY INDEX | GRAVEL (%) | COARSE (%) | MEDIUM (%) | FINE (%) | SILT & CLAY (%) | IS CLASSIFICATION | (%) SWELLING INDEX | BULK DENSITY (| DRY DENSITY (g/cc) | TYPE OF TEST | COHESION INTERCEPT (t/sqm) | ANGLE OF INTERNAL FRICTION (⁰) |
| | Test M | ethod | | IS 2720 (Part 2) | I IS 2720 (Part 5) IS 2720 (Part 4) IS 1498 IS 9736 (Part 2) IS 2720 (Part 11 / 13) | | | | | | | | | | | | / 13) | | |
| TP-1 | 1.5 | ı | DS | | Soil Stratum Excavated upto Bottom of Cellar Level | | | | | | | | | | | | | | |
| TP-1 | 3.0 | ı | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cell | ar Level | | | | | |
| TP-1 | 4.5 | ı | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cell | ar Level | | | | | |
| TP-1 | 6.0 | ı | DS | | | | | | C | ompletely W | eathered | Rock to \ | Weathere | d Rock | | | | | |
| TP-1 | 7.5 | ı | DS | | Completely Weathered Rock to Weathered Rock | | | | | | | | | | | | | | |
| TP-1 | 9.0 | - | DS | | | _ | | | C | ompletely W | eathered | Rock to \ | Weathere | d Rock | _ | _ | _ | | |

TABLE-5: SUMMARY OF LABORATORY TEST RESULTS OF SOIL SAMPLES

PROJECT: GEOTECHNICAL INVESTIGATION FOR PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENTS FOR M/S. MODI REALTY POCHARAM LLP AT POCHARAM, HYDERABAD

| N O | | | | (% | | | | | GRAIN SIZ | E DISTRIBU | JTION | | | | | | | SHEAR TE | ST |
|----------------------|-----------|----------|----------------|-----------------------------------|--|-------------------|------------------|---------------|---------------|---------------|-------------|-----------------------|-------------------|-----------------------|----------------|--------------------|--------------|-------------------------------|--------------------------------|
| PIT | | | ?LE | LNE | _ | ⊨ | Ä | | | SAND | ı | | NO. | ËX | (a/cc) | g/cc) | | EPT | AL |
| BOREHOLE / TRAIL PIT | DEPTH (m) | ON / LAS | TYPE OF SAMPLE | (%) WOISTURE CONTENT (%) (Part 2) | LIQUID LIMIT (%) | PLASTIC LIMIT (%) | PLASTICITY INDEX | GRAVEL (%) | COARSE (%) | MEDIUM (%) | FINE (%) | SILT & CLAY (%) | IS CLASSIFICATION | (%) SWELLING INDEX | BULK DENSITY (| DRY DENSITY (g/cc) | TYPE OF TEST | COHESION INTERCEPT (t/sqm) | ANGLE OF INTERNAL FRICTION (°) |
| | Test M | ethod | | | IS 2720 (Part 5) IS 2720 (Part 4) IS: 1498 IS 2720 (Part 40) SP 36 (Part 2) IS 2720 (Part 11 / 13) | | | | | | | | | | | / 13) | | | |
| TP-2 | 1.5 | ı | DS | | Soil Stratum Excavated upto Bottom of Cellar Level | | | | | | | | | | | | | | |
| TP-2 | 3.0 | ı | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cell | ar Level | | | | | |
| TP-2 | 4.5 | ı | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botte | om of Cell | ar Level | | | | | |
| TP-2 | 6.0 | - | DS | | | | | | C | ompletely W | eathered | Rock to | Weathered | d Rock | | | | | |
| TP-2 | 7.5 | 1 | DS | | Completely Weathered Rock to Weathered Rock | | | | | | | | | | | | | | |
| TP-2 | 9.0 | 1 | DS | | | | | | C | ompletely W | eathered | Rock to | Weathere | d Rock | | | | | |

TABLE-6: SUMMARY OF LABORATORY TEST RESULTS OF SOIL SAMPLES

PROJECT: GEOTECHNICAL INVESTIGATION FOR PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENTS FOR M/S. MODI REALTY POCHARAM LLP AT POCHARAM, HYDERABAD

| . 0 | | | | (%) | | | | | GRAIN SIZ | E DISTRIBL | ITION | | | | | | | SHEAR TE | ST |
|-------------------------|-----------|----------|----------------|----------------------|---|-------------------|------------------|---------------|---------------|---------------|-------------|-----------------------|-------------------|---------------------|----------------|--------------------|--------------|-------------------------------|-----------------------------------|
| PIT | | | ?LE |) TNE | _ | l _⊨ | OEX | | | SAND | | | NO. | INDEX | (a/cc) | g/cc) | | EPT | AL |
| BOREHOLE / TRAIL PIT NO | DEРТН (m) | au / Tas | TYPE OF SAMPLE | MOISTURE CONTENT (%) | LIQUID LIMIT (%) | PLASTIC LIMIT (%) | PLASTICITY INDEX | GRAVEL (%) | COARSE (%) | MEDIUM (%) | FINE (%) | SILT & CLAY (%) | IS CLASSIFICATION | (%) SWELLING IND | BULK DENSITY (| DRY DENSITY (g/cc) | TYPE OF TEST | COHESION INTERCEPT (t/sqm) | ANGLE OF INTERNAL FRICTION (°) |
| | Test M | ethod | | IS 2720 (Part 2) | I IS 2720 (Part 5) IS 2720 (Part 4) IS 1498 SP 36 (Part 2) IS 2720 (Part 11 / 13) | | | | | | | | | | | | / 13) | | |
| TP-3 | 1.5 | - | DS | | Soil Stratum Excavated upto Bottom of Cellar Level | | | | | | | | | | | | | | |
| TP-3 | 3.0 | - | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cell | ar Level | | | | | |
| TP-3 | 4.5 | 1 | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cell | ar Level | | | | | |
| TP-3 | 6.0 | - | DS | | | | | | C | ompletely W | eathered | Rock to \ | Neathere | d Rock | | | | | |
| TP-3 | 7.5 | ı | DS | | | | | | C | ompletely W | eathered | Rock to \ | Neathere | d Rock | | | | | |
| TP-3 | 9.0 | - | DS | | | | | | C | ompletely W | eathered | Rock to \ | Weathered | d Rock | | | | | |

TABLE-7: SUMMARY OF LABORATORY TEST RESULTS OF SOIL SAMPLES

PROJECT: GEOTECHNICAL INVESTIGATION FOR PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENTS FOR M/S. MODI REALTY POCHARAM LLP AT POCHARAM, HYDERABAD

| Ŏ. | | | | (% | | | | | GRAIN SIZ | E DISTRIBU | JTION | | | | | | | SHEAR TE | ST | |
|----------------------|-------------|----------|----------------|----------------------|---|-------------------|------------------|------------------|---------------|---------------|-------------|-----------------------|-------------------|-------------------------------------|--------------|--------------------|------------------------|-------------------------------|-----------------------------------|--|
| PIT | | | ?LE | LNE | _ | PLASTIC LIMIT (%) | PLASTICITY INDEX | | SAND | | ı | | NOI | INDEX | (a/cc) | g/cc) | | EPT | AL | |
| BOREHOLE / TRAIL PIT | DEРТН (m) | SPT / UD | TYPE OF SAMPLE | MOISTURE CONTENT (%) | LIQUID LIMIT (%) | | | GRAVEL (%) | COARSE (%) | MEDIUM (%) | FINE (%) | SILT & CLAY (%) | IS CLASSIFICATION | SWELLING INI | BULK DENSITY | DRY DENSITY (g/cc) | TYPE OF TEST | COHESION INTERCEPT (t/sqm) | ANGLE OF INTERNAL FRICTION (°) | |
| | Test Method | | | | IS | 2720 (Par | t 5) | IS 2720 (Part 4) | | | | | | IS 2720 (Part 40) SP 36 (Part 2) | | | IS 2720 (Part 11 / 13) | | | |
| TP-4 | 1.5 | ı | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cella | ar Level | | | | | | |
| TP-4 | 3.0 | ı | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cella | ar Level | | | | | | |
| TP-4 | 4.5 | 1 | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cella | ar Level | | | | | | |
| TP-4 | 6.0 | - | DS | | Completely Weathered Rock to Weathered Rock | | | | | | | | | | | | | | | |
| TP-4 | 7.5 | ı | DS | | Completely Weathered Rock to Weathered Rock | | | | | | | | | | | | | | | |
| TP-4 | 9.0 | ı | DS | | Completely Weathered Rock to Weathered Rock | | | | | | | | | | | | | | | |

TABLE-8: SUMMARY OF LABORATORY TEST RESULTS OF SOIL SAMPLES

PROJECT: GEOTECHNICAL INVESTIGATION FOR PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENTS FOR M/S. MODI REALTY POCHARAM LLP AT POCHARAM, HYDERABAD

| Ŏ. | | | | (%) | | | | | GRAIN SIZ | E DISTRIBL | JTION | | | | | | | SHEAR TE | ST | |
|------------------|--------------------------------------|--------|----------------|----------------------|---|-------------------|------------------|------------------|---------------|---------------|-------------|-----------------------|-------------------|----------------------|--------------|--------------------|------------------------|-------------------------------|-----------------------------------|--|
| PIT | | | ٦LE |) TN | | ⊨ | PLASTICITY INDEX | GRAVEL (%) | | SAND | | | NO! | INDEX | (a/cc) | g/cc) | | EPT | AL | |
| BOREHOLE / TRAIL | BOREHOLE / TRAIL PIT NO DEPTH (m) | SPT/UD | TYPE OF SAMPLE | MOISTURE CONTENT (%) | LIQUID LIMIT (%) | PLASTIC LIMIT (%) | | | COARSE (%) | MEDIUM (%) | FINE (%) | SILT & CLAY (%) | IS CLASSIFICATION | SWELLING IND (%) | BULK DENSITY | DRY DENSITY (g/cc) | TYPE OF TEST | COHESION INTERCEPT (t/sqm) | ANGLE OF INTERNAL FRICTION (°) | |
| | Test M | ethod | | IS 2720 (Part 2) | IS | 2720 (Par | t 5) | IS 2720 (Part 4) | | | | | | IS 2720 (Part 40) | SP 36 | (Part 2) | IS 2720 (Part 11 / 13) | | | |
| TP-5 | 1.5 | ı | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cell | ar Level | | | | | | |
| TP-5 | 3.0 | ı | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cell | ar Level | | | | | | |
| TP-5 | 4.5 | 1 | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cell | ar Level | | | | | | |
| TP-5 | 6.0 | - | DS | | Completely Weathered Rock to Weathered Rock | | | | | | | | | | | | | | | |
| TP-5 | 7.5 | ı | DS | | Completely Weathered Rock to Weathered Rock | | | | | | | | | | | | | | | |
| TP-5 | 9.0 | - | DS | | Completely Weathered Rock to Weathered Rock | | | | | | | | | | | | | | | |

TABLE-9: SUMMARY OF LABORATORY TEST RESULTS OF SOIL SAMPLES

PROJECT: GEOTECHNICAL INVESTIGATION FOR PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENTS FOR M/S. MODI REALTY POCHARAM LLP AT POCHARAM, HYDERABAD

| . 0 | | | | (% | | | | | GRAIN SIZ | E DISTRIBL | ITION | | | | | | | SHEAR TE | ST |
|-------------------------|-----------|----------|----------------|----------------------|---|-------------------|------------------|------------------|---------------|---------------|-------------|-----------------------|-------------------|----------------------|--------------|--------------------|--------------|-------------------------------|-----------------------------------|
| PIT | | | ٦LE |) TNE | _ | PLASTIC LIMIT (%) | PLASTICITY INDEX | GRAVEL (%) | SAND | | | | | INDEX | (a/cc) | g/cc) | | EPT | AL |
| BOREHOLE / TRAIL PIT NO | DEРТН (m) | SPT / UD | TYPE OF SAMPLE | MOISTURE CONTENT (%) | LIQUID LIMIT (%) | | | | COARSE (%) | MEDIUM (%) | FINE (%) | SILT & CLAY (%) | IS CLASSIFICATION | SWELLING IN (%) | BULK DENSITY | DRY DENSITY (g/cc) | TYPE OF TEST | COHESION INTERCEPT ('Úsqm) | ANGLE OF INTERNAL FRICTION (°) |
| | Test M | ethod | | IS 2720 (Part 2) | IS | 2720 (Pari | t 5) | IS 2720 (Part 4) | | | | | | IS 2720 (Part 40) | SP 36 | (Part 2) | IS | 3 2720 (Part 11 | / 13) |
| TP-6 | 1.5 | - | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cell | ar Level | | | | | |
| TP-6 | 3.0 | - | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cell | ar Level | | | | | |
| TP-6 | 4.5 | ı | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cell | ar Level | | | | | |
| TP-6 | 6.0 | - | DS | | Completely Weathered Rock to Weathered Rock | | | | | | | | | | | | | | |
| TP-6 | 7.5 | ı | DS | | Completely Weathered Rock to Weathered Rock | | | | | | | | | | | | | | |
| TP-6 | 9.0 | - | DS | | Completely Weathered Rock to Weathered Rock | | | | | | | | | | | | | | |

TABLE-10: SUMMARY OF LABORATORY TEST RESULTS OF SOIL SAMPLES

PROJECT: GEOTECHNICAL INVESTIGATION FOR PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENTS FOR M/S. MODI REALTY POCHARAM LLP AT POCHARAM, HYDERABAD

| NO. | | | | (%) | | | | | GRAIN SIZE DISTRIBUTION | | | | | | | | | SHEAR TE | ST | |
|----------------------|-----------|----------|----------------|----------------------|---|-------------------|------------------|------------------|-------------------------|---------------|-------------|-----------------------|-------------------|----------------------|--------------|--------------------|---|----------|-----------------------------------|--|
| PIT | | | ?LE |) TNE | _ | ⊨ | PLASTICITY INDEX | | SAND | | ı | | | INDEX | (a/cc) | g/cc) | | EPT | AL | |
| BOREHOLE / TRAIL PIT | DEРТН (m) | ON / 1AS | TYPE OF SAMPLE | MOISTURE CONTENT (%) | LIQUID LIMIT (%) | PLASTIC LIMIT (%) | | GRAVEL (%) | COARSE (%) | MEDIUM (%) | FINE (%) | SILT & CLAY (%) | IS CLASSIFICATION | SWELLING INI | BULK DENSITY | DRY DENSITY (g/cc) | TYPE OF TEST COHESION INTERCEPT (t/sqm) | | ANGLE OF INTERNAL FRICTION (°) | |
| | Test M | ethod | | IS 2720 (Part 2) | IS | 2720 (Par | t 5) | IS 2720 (Part 4) | | | | | | IS 2720 (Part 40) | SP 36 | (Part 2) | IS 2720 (Part 11 / 13) | | | |
| TP-7 | 1.5 | - | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cell | ar Level | | | | | | |
| TP-7 | 3.0 | - | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cell | ar Level | | | | | | |
| TP-7 | 4.5 | 1 | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cell | ar Level | | | | | | |
| TP-7 | 6.0 | - | DS | | Completely Weathered Rock to Weathered Rock | | | | | | | | | | | | | | | |
| TP-7 | 7.5 | ı | DS | | Completely Weathered Rock to Weathered Rock | | | | | | | | | | | | | | | |
| TP-7 | 9.0 | - | DS | | Completely Weathered Rock to Weathered Rock | | | | | | | | | | | | | | | |

TABLE-11: SUMMARY OF LABORATORY TEST RESULTS OF SOIL SAMPLES

PROJECT: GEOTECHNICAL INVESTIGATION FOR PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENTS FOR M/S. MODI REALTY POCHARAM LLP AT POCHARAM, HYDERABAD

| NO. | | | | (%) | | | | | GRAIN SIZ | E DISTRIBU | JTION | | | | | | | SHEAR TEST | | |
|----------------------|-----------|--------|---------------------|----------------------|---|-------------------|------------------|------------|---------------|---------------|-------------|-----------------------|-------------------------------------|--------------|--------------|------------------------|--------------|-------------------------------|--------------------------------|--|
| PIT | | | ?LE |) TNE | _ | ⊨ | DEX | | SAND | | ī | | | INDEX | (a/cc) | g/cc) | | EPT | AL | |
| BOREHOLE / TRAIL PIT | DEРТН (m) | SPT/UD | TYPE OF SAMPLE | MOISTURE CONTENT (%) | LIQUID LIMIT (%) | PLASTIC LIMIT (%) | PLASTICITY INDEX | GRAVEL (%) | COARSE (%) | MEDIUM (%) | FINE (%) | SILT & CLAY (%) | IS CLASSIFICATION | SWELLING INI | BULK DENSITY | DRY DENSITY (g/cc) | TYPE OF TEST | COHESION INTERCEPT (t/sqm) | ANGLE OF INTERNAL FRICTION (°) | |
| Test Method | | | IS 2720 (Part 2) | IS | 2720 (Par | t 5) | IS 2720 (Part 4) | | | | | | IS 2720 (Part 40) SP 36 (Part 2) | | | IS 2720 (Part 11 / 13) | | | | |
| TP-8 | 1.5 | 1 | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cell | ar Level | | | | | | |
| TP-8 | 3.0 | - | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botto | om of Cell | ar Level | | | | | | |
| TP-8 | 4.5 | ı | DS | | | | | | Soi | l Stratum Ex | cavated | upto Botte | om of Cell | ar Level | | | | | | |
| TP-8 | 6.0 | ı | DS | | Completely Weathered Rock to Weathered Rock | | | | | | | | | | | | | | | |
| TP-8 | 7.5 | 1 | DS | | Completely Weathered Rock to Weathered Rock | | | | | | | | | | | | | | | |
| TP-8 | 9.0 | - | DS | | Completely Weathered Rock to Weathered Rock | | | | | | | | | | | | | | | |

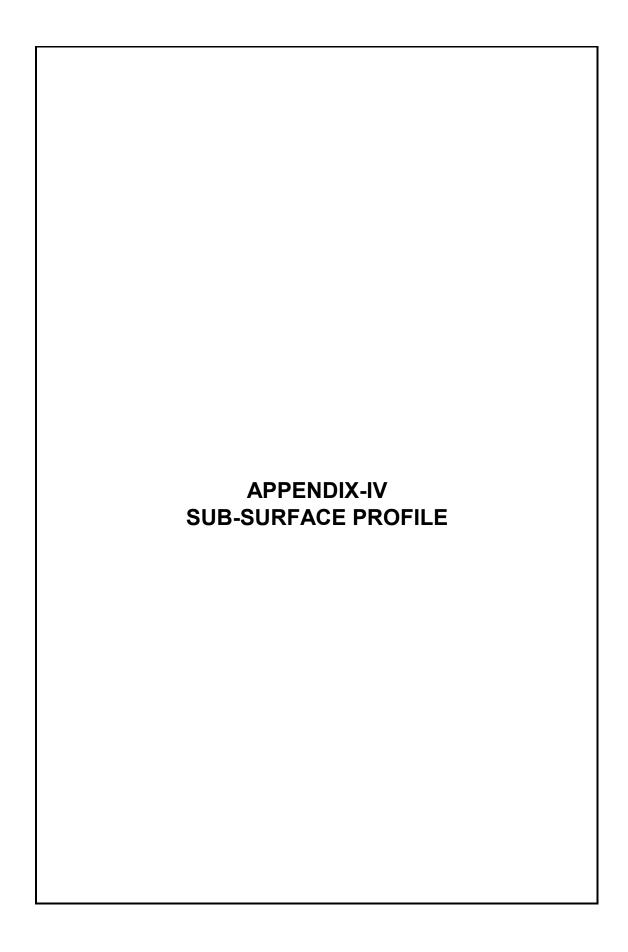


FIGURE-1: SUB-SURFACE PROFILE

PROJECT: GEOTECHNICAL INVESTIGATION FOR PROPOSED CONSTRUCTION OF

RESIDENTIAL APARTMENTS FOR M/S. MODI REALTY POCHARAM LLP AT POCHARAM, HYDERABAD

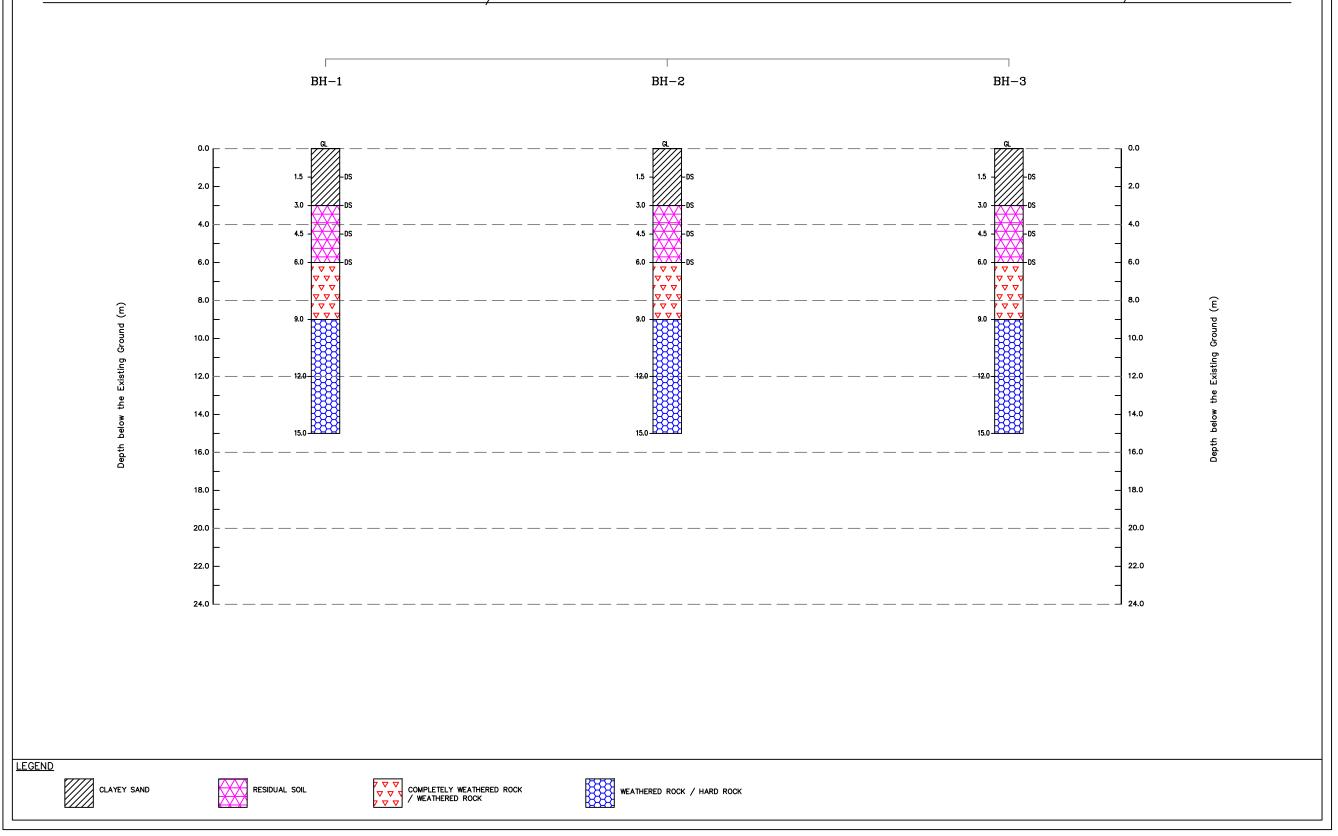


FIGURE-2: SUB-SURFACE PROFILE

PROJECT: GEOTECHNICAL INVESTIGATION FOR PROPOSED CONSTRUCTION OF

RESIDENTIAL APARTMENTS FOR M/S. MODI REALTY POCHARAM LLP AT POCHARAM, HYDERABAD

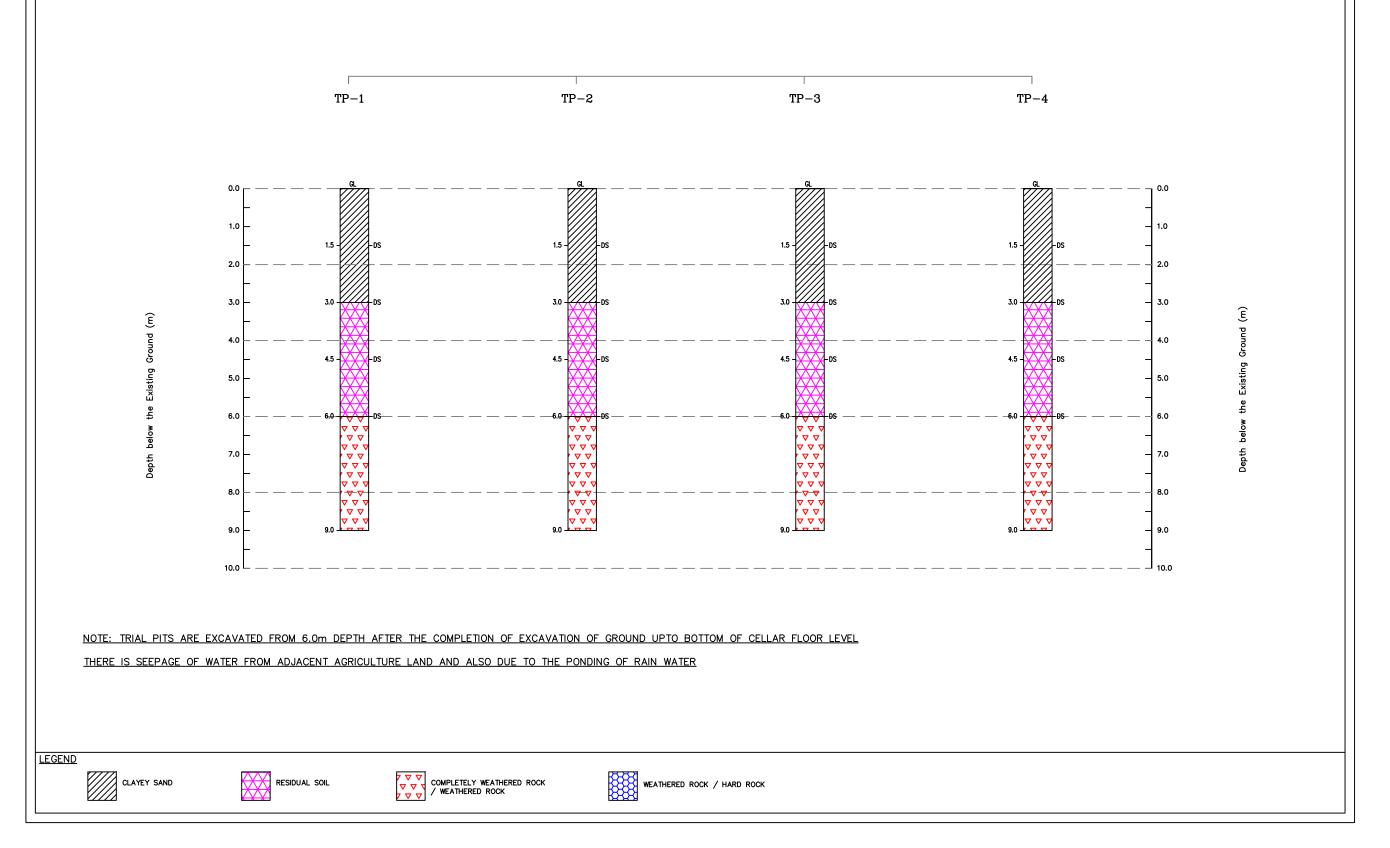
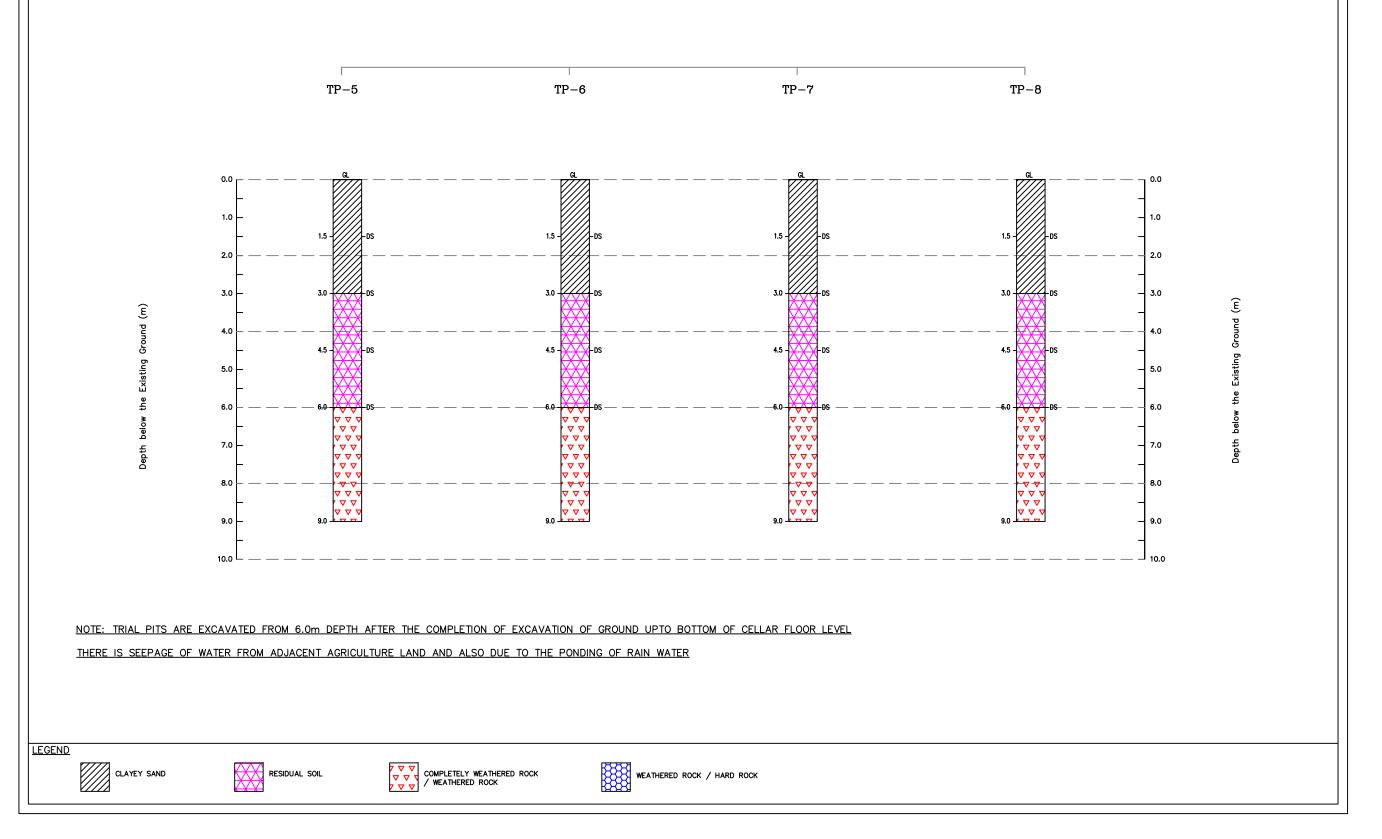


FIGURE-3: SUB-SURFACE PROFILE

PROJECT: GEOTECHNICAL INVESTIGATION FOR PROPOSED CONSTRUCTION OF

RESIDENTIAL APARTMENTS FOR M/S. MODI REALTY POCHARAM LLP AT POCHARAM, HYDERABAD



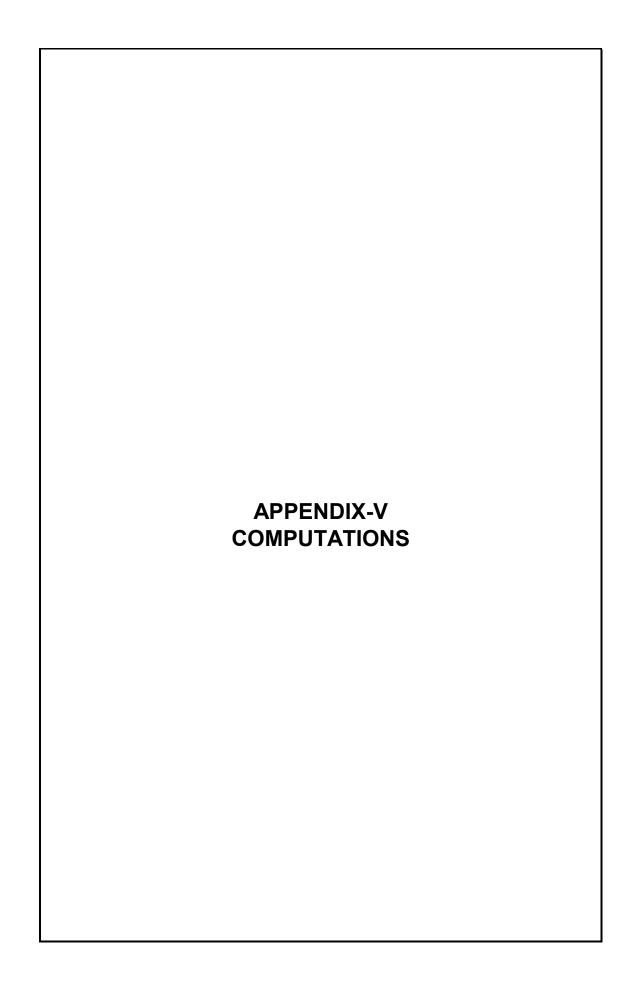


Table-1: COMPUTATION OF NET SAFE BEARING CAPACITY AS PER IS: 6403

PROJECT: GEOTECHNICAL INVESTIGATION FOR PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENTS FOR M/S. MODI REALTY POCHARAM LLP AT POCHARAM, HYDERABAD

Depth of Foundation = 1.5m below the bottom of cellar ground level

| 1 | Type of Failure | | Genral Shear Failure | |
|----|--|--|--|-------|
| 2 | Foundation Strata | | Completely Weathered Rock / Weathered Rock | |
| 3 | Type of Foundation | | Isolated | |
| 4 | Width of foundation (B) | | 2.0 | m |
| 5 | Length of foundation (L) | | 2.0 | m |
| 6 | Depth of Foundation, D _f | | 1.5 | m |
| 7 | Average SPT corrected N" Value below the foundation depth and Zone of influence of 1.5 to 2.0B m depth. | | >100 | |
| 8a | Angle of Internal Friction by correlating SPT N" Value with Phi values as per Figure-1 of IS: 6403, ϕ | | 40.0 | |
| 8b | Limiting Angle of Internal Friction taking Gra | in Size and other factors into consideration, $\boldsymbol{\phi}$ | 39.0 | deg |
| 9 | Cohesive Strength of Soil, c | | 0.0 | T/sqm |
| 10 | Unit Weight of Overburden Soil, γ | | 1.85 | T/cum |
| 11 | Over burden pressure soil | | 1.28 | |
| 12 | Bearing Capacity Factors Range | | | |
| 13 | Bearing Capacity Factor, N _c (Table 1 of 6403) | | 69.47 | |
| 14 | Bearing Capacity Factor, N _q (Table 1 of 6403 | 3) | 58.02 | |
| 15 | Bearing Capacity Factor, N _y (Table 1 of 6403 | 3) | 97.13 | |
| 16 | Depth Factor, d _c | = 1 +($(0.2 \times D_f/B) \times Sqrt(N_{\phi})$) | 1.31 | |
| 17 | Depth Factor, d _q | = (1 if φ < 10) else, 1 + ((0.1 x D _f / B) x Sqrt(N $_{\varphi}$)) | 1.16 | |
| 18 | Depth Factor, d _y | = (1 if φ < 10) else, 1 + ((0.1 x D _f / B) x Sqrt(N $_{\varphi}$)) | 1.16 | |
| 19 | Shape Factor, s _c | Constant = 1 + 0.2* B / L | 1.20 | |
| 20 | Shape Factor, s _q | Constant = 1 + 0.2* B / L | 1.20 | |
| 21 | Shape Factor, s _y | Constant = 1 - 0.4* B / L | 0.60 | |
| 22 | Inclination Factor, i _c | $= (1 - \alpha / 90)^2$ | 1.00 | |
| 23 | Inclination Factor, i _q | $= (1 - \alpha / 90)^2$ | 1.00 | |
| 24 | Inclination Factor, i _y | $= (1 - \alpha / \phi)^2$ | 1.00 | |
| 25 | Water Table Correction Factor, W' | Constant | 0.50 | |
| 26 | Ultimate Bearing Capacity Component, UBC | $= c N_c s_c d_c i_c$ | 0.00 | T/sqm |
| 27 | Ultimate Bearing Capacity Component, UBC | '''' | 101.20 | T/sqm |
| 28 | Ultimate Bearing Capacity Component, UBC | $= 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma} W'$ | 62.53 | T/sqm |
| 29 | Net Ultimate Bearing Capacity, UBC | = UBC1 + UBC2 + UBC3 | 163.73 | T/sqm |
| 30 | Net SBC with Factor of Safety of 2.5 | = UBC / 2.5 | | T/sqm |
| | | Say | 65.5 | T/sqm |

Table-2: COMPUTATION OF NET SAFE BEARING CAPACITY AS PER IS: 6403

PROJECT: GEOTECHNICAL INVESTIGATION FOR PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENTS FOR M/S. MODI REALTY POCHARAM LLP AT POCHARAM, HYDERABAD

Depth of Foundation = 2.0m below the bottom of cellar ground level

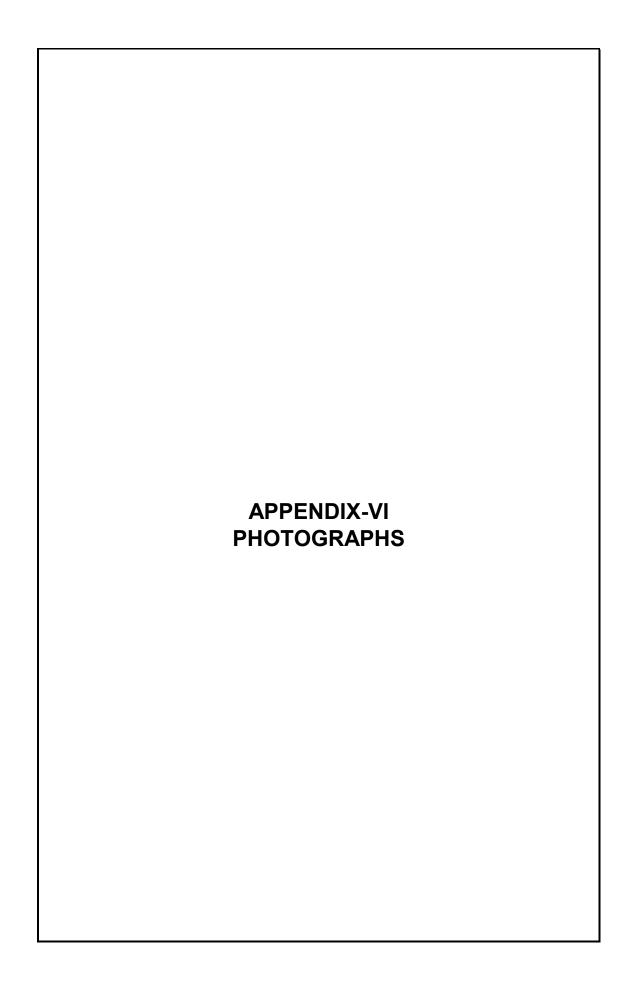
| 1 | Type of Failure | | Genral Shear Failure | |
|----|--|--|--|-------|
| 2 | Foundation Strata | | Completely Weathered Rock / Weathered Rock | |
| 3 | Type of Foundation | | Isolated | |
| 4 | Width of foundation (B) | | 2.0 | m |
| 5 | Length of foundation (L) | | 2.0 | m |
| 6 | Depth of Foundation, D _f | | 2.0 | m |
| 7 | Average SPT corrected N" Value below the foundation depth and Zone of influence of 1.5 to 2.0B m depth. | | >100 | |
| 8a | Angle of Internal Friction by correlating SPT N" Value with Phi values as per Figure-1 of IS: 6403, ϕ | | 40.0 | |
| 8b | Limiting Angle of Internal Friction taking Gra | in Size and other factors into consideration, $\boldsymbol{\phi}$ | 39.0 | deg |
| 9 | Cohesive Strength of Soil, c | | 0.0 | T/sqm |
| 10 | Unit Weight of Overburden Soil, γ | | 1.85 | T/cum |
| 11 | Over burden pressure soil | | 1.70 | |
| 12 | Bearing Capacity Factors Range | | | |
| 13 | Bearing Capacity Factor, N₂ (Table 1 of 6403) | | 69.47 | |
| 14 | Bearing Capacity Factor, N _q (Table 1 of 6403 | 3) | 58.02 | |
| 15 | Bearing Capacity Factor, N _y (Table 1 of 6403 | 3) | 97.13 | |
| 16 | Depth Factor, d _c | = 1 +($(0.2 \times D_f/B) \times Sqrt(N_{\phi})$) | 1.42 | |
| 17 | Depth Factor, d _q | = (1 if φ < 10) else, 1 + ((0.1 x D_f/ B) x Sqrt(N_ $_{\varphi}))$ | 1.21 | |
| 18 | Depth Factor, d _y | = (1 if φ < 10) else, 1 + ((0.1 x D _f / B) x Sqrt(N $_{\varphi}$)) | 1.21 | |
| 19 | Shape Factor, s _c | Constant = 1 + 0.2* B / L | 1.20 | |
| 20 | Shape Factor, s _q | Constant = 1 + 0.2* B / L | 1.20 | |
| 21 | Shape Factor, s _y | Constant = 1 - 0.4* B / L | 0.60 | |
| 22 | Inclination Factor, i _c | $= (1 - \alpha / 90)^2$ | 1.00 | |
| 23 | Inclination Factor, i _q | $= (1 - \alpha / 90)^2$ | 1.00 | |
| 24 | Inclination Factor, i _y | $= (1 - \alpha / \phi)^2$ | 1.00 | |
| 25 | Water Table Correction Factor, W' | Constant | 0.50 | |
| 26 | Ultimate Bearing Capacity Component, UBC | $= c N_c s_c d_c i_c$ | 0.00 | T/sqm |
| 27 | Ultimate Bearing Capacity Component, UBC | $= q (N_q-1) s_q d_q i_q$ | 140.75 | T/sqm |
| 28 | Ultimate Bearing Capacity Component, UBC | $= 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma} W'$ | 65.23 | T/sqm |
| 29 | Net Ultimate Bearing Capacity, UBC | = UBC1 + UBC2 + UBC3 | 205.98 | T/sqm |
| 30 | Net SBC with Factor of Safety of 2.5 | = UBC / 2.5 | 82.39 | T/sqm |
| | | Say | 82.4 | T/sqm |

Table-3: COMPUTATION OF NET SAFE BEARING CAPACITY AS PER IS: 6403

PROJECT: GEOTECHNICAL INVESTIGATION FOR PROPOSED CONSTRUCTION OF RESIDENTIAL APARTMENTS FOR M/S. MODI REALTY POCHARAM LLP AT POCHARAM, HYDERABAD

Depth of Foundation = 2.5m below the bottom of cellar ground level

| | Type of Failure | | Genral Shear Failure | |
|----|---|--|--|-------|
| 2 | Foundation Strata | | Completely Weathered Rock / Weathered Rock | |
| 3 | Type of Foundation | | Isolated | |
| 4 | Width of foundation (B) | | 2.0 | m |
| 5 | Length of foundation (L) | | 2.0 | m |
| 6 | Depth of Foundation, D _f | | 2.5 | m |
| | Average SPT corrected N" Value below the foundation depth and Zone of influence of 1.5 to 2.0B m depth. | | >100 | |
| | Angle of Internal Friction by correlating SPT 6403, φ | N" Value with Phi values as per Figure-1 of IS: | 40.0 | |
| 8b | Limiting Angle of Internal Friction taking Grai | n Size and other factors into consideration, $\boldsymbol{\phi}$ | 39.0 | deg |
| 9 | Cohesive Strength of Soil, c | | 0.0 | T/sqm |
| 10 | Unit Weight of Overburden Soil, γ | | 1.85 | T/cum |
| 11 | Over burden pressure soil | | 2.13 | |
| 12 | Bearing Capacity Factors Range | | | |
| 13 | Bearing Capacity Factor, N _c (Table 1 of 6403) | | 69.47 | |
| 14 | Bearing Capacity Factor, N _q (Table 1 of 6403) | | 58.02 | |
| 15 | Bearing Capacity Factor, N_y (Table 1 of 6403 | 3) | 97.13 | |
| 16 | Depth Factor, d _c | = 1 +($(0.2 \times D_{f}/B) \times Sqrt(N_{\phi})$) | 1.52 | |
| 17 | Depth Factor, d _q | = (1 if ϕ < 10) else, 1 + ((0.1 x D _f / B) x Sqrt(N $_{\phi}$)) | 1.26 | |
| 18 | Depth Factor, d _y | = (1 if ϕ < 10) else, 1 + ((0.1 x D _f / B) x Sqrt(N $_{\phi}$)) | 1.26 | |
| 19 | Shape Factor, s _c | Constant = 1 + 0.2* B / L | 1.20 | |
| 20 | Shape Factor, s _q | Constant = 1 + 0.2* B / L | 1.20 | |
| 21 | Shape Factor, s _y | Constant = 1 - 0.4* B / L | 0.60 | |
| 22 | Inclination Factor, i _c | $= (1 - \alpha / 90)^2$ | 1.00 | |
| 23 | Inclination Factor, i _q | $= (1 - \alpha / 90)^2$ | 1.00 | |
| 24 | Inclination Factor, i _y | $= (1 - \alpha / \phi)^2$ | 1.00 | |
| 25 | Water Table Correction Factor, W' | Constant | 0.50 | |
| 26 | Ultimate Bearing Capacity Component, UBC | $= c N_c s_c d_c i_c$ | 0.00 | T/sqm |
| 27 | Ultimate Bearing Capacity Component, UBC | $= q (N_q-1) s_q d_q i_q$ | 183.21 | T/sqm |
| 28 | Ultimate Bearing Capacity Component, UBC | $= 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma} W'$ | 67.92 | T/sqm |
| 29 | Net Ultimate Bearing Capacity, UBC | = UBC1 + UBC2 + UBC3 | 251.13 | T/sqm |
| 30 | Net SBC with Factor of Safety of 2.5 | = UBC / 2.5 | 100.45 | T/sqm |
| | | Say | 100.5 | T/sqm |



PHOTOGRAPHS



Photograph - 1: Excavated Trial Pit TP-1 Location





Photograph - 3: Excavated Trial Pit TP-3 Location



Photograph - 4: Excavated Trial Pit TP-4 Location



Photograph - 5: Excavated Trial Pit TP-5 Location



Photograph - 6: Excavated Trial Pit TP-6 Location



Photograph - 7: Excavated Trial Pit TP-7 Location



Photograph - 8: Excavated Trial Pit TP-8 Location